

## 2. DEFINITIONS & PROPERTIES OF SOIL

- \* Partially saturated soil : Solids + water + air (3 phase system)
- Fully saturated soil : Solids + water. (2 phase system)
- Dry soil : Solids + air
- \* Frozen soil : Solids + water + ice + air. (4 phase system)

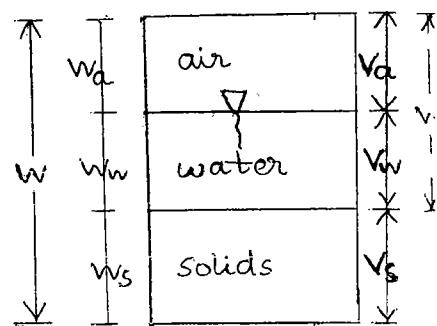
$V_s \rightarrow$  volume of solids.

$V \rightarrow$  volume of soil.

$V_v \rightarrow$  volume of voids.

$$V_v = V_w + V_a.$$

$$V = V_s + V_w + V_a.$$



Phase Diagram  
OR  
Block Diagram

### \* Void Ratio, e

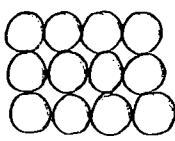
$$e = \frac{V_v}{V_s}$$

Range :- more than zero, it can have any value (no limit).

- For coarse grained soil,  $e < 1$  generally.

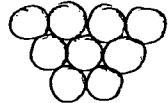
- For fine grained soil,  $e > 1$  generally

- The 'e' of FG soil is generally more than that of a coarse grained soil.



Cubical array  
(loosest state)

$$e_{max} = 0.91$$



Diagonal array  
(densest state)

$$e_{max} = 0.35$$

\* Specific Gravity of Soil Solids, G

$$G = \frac{\gamma_s}{\gamma_w} ; \quad \gamma_w \rightarrow \text{distilled water (pure water)}$$

Also called 'True Specific gravity of Soil'

For soils, G : 2.60 — 2.85 generally.  
(inorganic)

\* Mass Specific Gravity of Soil. (or) Bulk Sp. Gr. of soil  
(or) Apparent Specific Gravity of soil, Gm.

$$\text{For a dry soil, } G_m = \frac{\gamma_d}{\gamma_w}$$

$$\text{For a partially saturated soil, } G_m = \frac{\gamma}{\gamma_w}$$

$$\text{For a fully saturated soil, } G_m = \frac{\gamma_{sat}}{\gamma_w}$$

$$G_m < G$$

For cement :-

True sp. gravity, G = 3.15

Apparent sp. gravity, Gm = 1.44

For cement,  $\gamma_s = 3150 \text{ kgf/m}^3$

$\gamma_d = 1440 \text{ kgf/m}^3$

$n \approx 44\%$

Important Relationships :

$$1. e = \frac{\omega G_1}{S_r}$$

$$4. \gamma_d = \frac{\gamma_w G}{1+e}$$

$$7. \gamma = \gamma_d + S_r(\gamma_{sat} - \gamma)$$

$$2. \gamma = \gamma_w \left( \frac{G + e S_r}{1+e} \right)$$

$$5. \gamma_d = \frac{\gamma}{1+\omega}$$

$$3. \gamma_{sat} = \gamma_w \left( \frac{G + e}{1+e} \right)$$

$$6. \gamma_d = \frac{(1-na)\gamma_w G}{1+\omega G}$$

\* Saturated unit weight of Soil,  $\gamma_{sat}$ .

5<sup>(4)</sup>

It is the bulk unit weight of soil in a saturated condition.  $\Rightarrow \gamma_{sat} > \gamma$

For partially saturated soil,  $\gamma_{sat}$  use  $\gamma$

For fully saturated soil, use  $\gamma_{sat}$ .

\* Dry unit weight of Soil,  $\gamma_d$ .

$$\gamma_d = \frac{w_s}{V}$$

It can be used irrespective of saturation level of soil.

\* Unit weight of Solids,  $\gamma_s$

$$\gamma_s = \frac{w_s}{V_s}$$

\* Submerged Unit Weight of Soil,  $\gamma_{sub}$  or  $\gamma'$

It is the submerged wt. of soil per unit volume of soil.

$$\gamma' = \gamma_{sat} - \gamma_w$$

$\left\{ \begin{array}{l} \gamma_{sat} \downarrow - \text{gravity force} \\ \gamma_w \uparrow - \text{buoyant force} \end{array} \right.$

Submerged weight of soil is based on Archimedes Principle.

$\gamma_w$  = unit weight of water

$$= 1 \text{ g/cc} = 1 \text{ ton/m}^3 = 1000 \text{ kgf/m}^3$$

$$= 9.81 \text{ kN/m}^3 \approx 10 \text{ kN/m}^3$$

$$\boxed{\gamma_s > \gamma_{sat} > \gamma_{bulk} > \gamma_{dry} > \gamma'}$$

For a given soil,  $\gamma_s$  remains a constant

Aug.  
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\* Porosity,  $n$  (also called "Percentage voids")

4 5

$$n = \frac{V_v}{V} \times 100$$

Range:  $0 < n < 100\%$  ( $V_v \neq 0$ ,  $\text{soil}, \therefore n \neq 0$ )

$$n = \frac{e}{1+e}$$

\* Degree of Saturation,  $S_r$

$$S_r = \frac{V_w}{V_v} \times 100$$

For a dry soil,  $S_r = \frac{0}{V_v} \times 100 = 0$  ( $V_w = 0$ )

For a saturated soil,  $S_r = \frac{V_v}{V_v} \times 100 = 100$  ( $V_w = V_v$ )

Range:  $0 \leq S_r \leq 100\%$

\* Air content,  $a_c$

$$a_c = \frac{V_a}{V_v}$$

For a saturated soil,  $a_c = \frac{0}{V_v} = 0$  ( $V_a = 0$ )

For a dry soil,  $a_c = \frac{V_v}{V_v} = 1$ . ( $V_a = V_v$ )

Range:  $0 \leq a_c \leq 1$

\* % air voids,  $n_a$

$$n_a = \frac{V_a}{V} \times 100$$

For a saturated soil, ( $V_a = 0$ ),  $n_a = 0$

For a dry soil,  $n_a = n$  ( $V_a = V_v$ )

Range:  $0 \leq n_a \leq n$

$$a_c + S_r = \frac{V_a}{V_v} + \frac{V_w}{V_v}$$

$$= \frac{V_a + V_w}{V_v} = \frac{V_v}{V_v} = 1.$$

$$\therefore a_c + S_r = 1$$

$$n.a_c = \frac{V_v}{V} \times \frac{V_a}{V_v} = \frac{V_a}{V} = n_a.$$

$$n.a_c = n_a$$

$w_s$  → weight of solids.

$w$  → weight of soil.

$w = w_s + w_w$  ( $w_a$  is negligible)

$w_d$  → <sup>total</sup> weight of soil in dry condition. ( $= w_s$ ).

\* Water Content,  $\omega$

$$\omega = \frac{w_w}{w_s} \times 100$$

For dry soil,  $\omega = \frac{0}{w_s} \times 100 = 0$

For a saturated soil,  $w > 0$ .

(i.e., water content <sup>can</sup> have any value greater than zero  
Sometimes  $> 100\%$ , sometimes  $< 100\%$ )

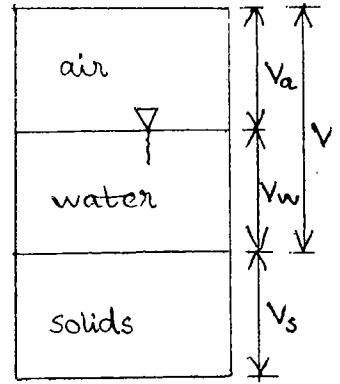
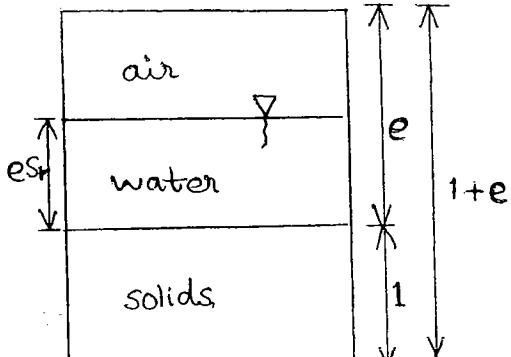
\* Bulk unit weight of Soil,

- it is the total weight of soil per unit volume of soil.

$$\gamma = \frac{w}{V}$$

In a unit phase diagram,  
lets take  $V_s = 1$

$$e = \frac{V_v}{V_s}$$



If  $V_s = 1$ , then

$$e = V_v$$

$$S_r = \frac{V_w}{V_v} \Rightarrow V_w = V_v \cdot S_r \\ = e \cdot S_r$$

① To derive  $n$  &  $e$  relationship:

$$n = \frac{V_v}{V} = \frac{e}{1+e}$$

② To derive  $e = \frac{\omega G_1}{S_r}$

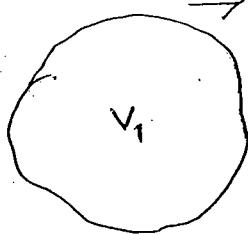
$$\omega = \frac{W_w}{W_s} = \frac{V_w \gamma_w}{V_s \cdot \gamma_s} = \frac{e S_r}{1 \cdot G}$$

$$\Rightarrow e = \frac{\omega G_1}{S_r}$$

③ To derive  $\gamma = \gamma_w \left( \frac{G + e S_r}{1 + e} \right)$

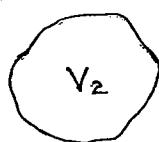
$$\gamma = \frac{W_s + W_w}{V} = \frac{\gamma_s W_s + \gamma_w V_w}{V} \\ = \frac{\gamma_s + \gamma_w e \cdot S_r}{1 + e}$$

$$\Rightarrow \gamma = \frac{\gamma_w (G + e S_r)}{1 + e}$$



Before compaction

$$e_1, \gamma_{d1}, W_s$$



After compaction

$$e_2, \gamma_{d2}, W_s$$

$$\frac{V_2}{V_1} = \frac{1+e_2}{1+e_1}$$

$$\therefore V \propto (1+e)$$

$$\gamma_d = \frac{w_s}{V} \Rightarrow \gamma_d \propto \frac{1}{V}$$

$$\frac{V_2}{V_1} = \frac{\gamma_{d1}}{\gamma_{d2}}$$

Q. Due to compaction, the void ratio of a soil reduced from 1 to 0.6. What is the % volume loss.

$$\frac{V_2}{V_1} = \frac{1+0.6}{1+1} = \frac{1.6}{2}$$

$$V_2 = 0.8 V_1$$

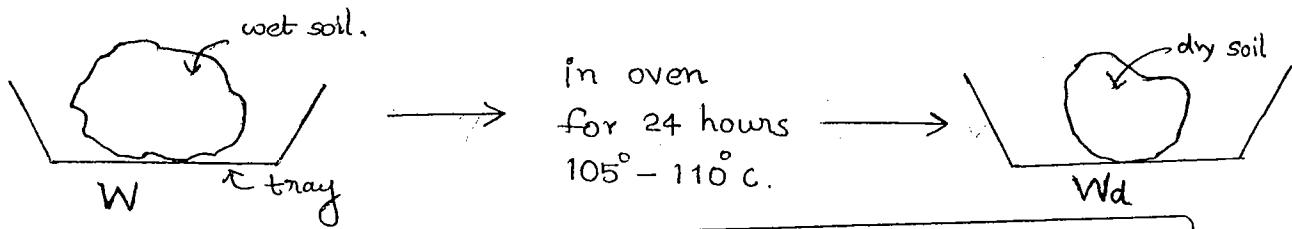
$\therefore$  vol. reduced to 80%.

vol. reduced by 20%  $\Rightarrow$  volume loss is 20%.

\* To find Water Content of Soil :

- a) Oven drying Method.  $\rightarrow$  most accurate method.
- b) Pycnometer Method  $\rightarrow$  can be used only if 'G' is known.
- c) Sand bath method  $\rightarrow$  quick field method. (approx. value).
- d) Calcium Carbide method  $\rightarrow$  quick method.
- e) Tension balance method  $\rightarrow$  to find w.c at different depths below G.L

a) Oven Drying method :



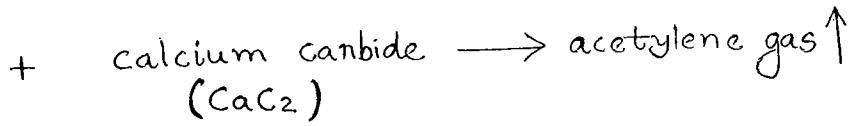
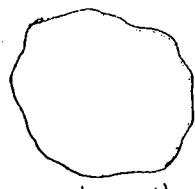
$w \rightarrow$  wt. of wet soil

$w_d =$  wt. of dry soil =  $w_s$

$$w = \frac{W_w}{W_s} \times 100 = \frac{W - W_d}{W_d} \times 100$$

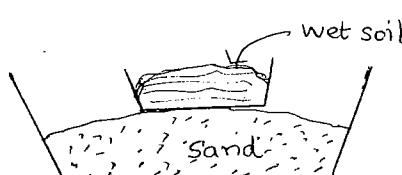
b) Calcium Carbide method.

7 (e)



Amount of acetylene gas produced is directly proportional to water content of soil.

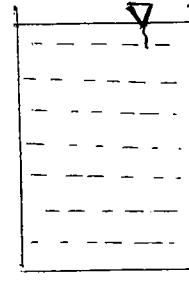
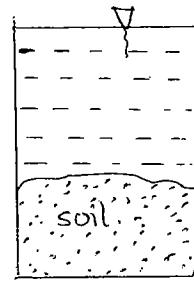
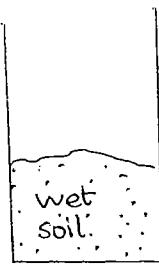
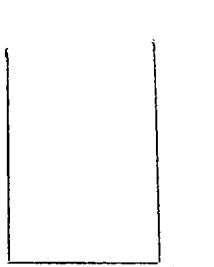
c) Sand Bath method.



It's a crude method used in the field.



d) Pycnometer Method

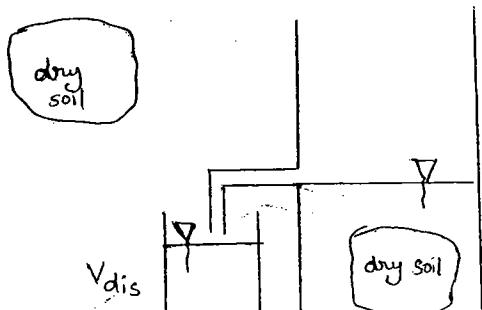


$$\text{Water content, } w = \left[ \frac{W_2 - W_1}{W_3 - W_4} \left( \frac{G-1}{G} \right) - 1 \right] \times 100$$

- To find Specific Gravity, G, replace second container with dry soil

$$G = \frac{W_2 - W_1}{(W_2 - W_1) - (W_3 - W_4)}$$

- To find volume:



$V_{\text{dis}}$  → volume of water displaced

- If dry soil is immersed, then

$$V_{\text{dis}} = V_s \text{ (vol. of solids)}$$

- If partially saturated soil is immersed,

$$V_{\text{dis}} = V_s + V_w$$

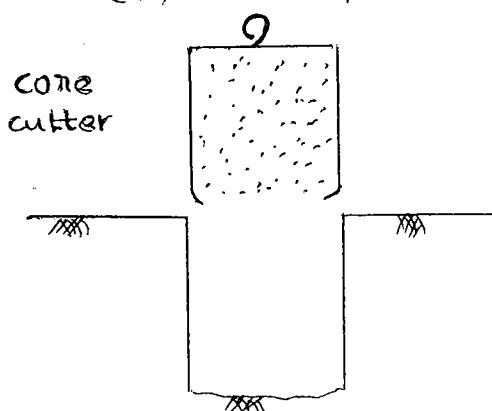
◎ Partially saturated soil with water coating, then

$$V_{dis} = \text{total volume of soil} = V + \text{vol. of wa}$$

\* To determine in-situ  $\gamma_d$  and  $e$

(i) Core Cutter Method. → suitable for clays only (cohesive).

(ii) Sand Replacement method. → suitable for any soil



$$\gamma = \frac{w}{V}$$

$$\gamma_d = \frac{\gamma}{1+w} = \frac{\gamma_w G}{1+e}$$

12<sup>th</sup> Aug,  
TUESDAY

P-8.

$$1. \quad V_a = \frac{V}{6}, \quad V_w = \frac{V}{3}$$

$$V_a + V_w = V_v$$

$$\therefore V_v = \frac{V}{6} + \frac{V}{3} = \frac{V}{2}$$

$$V_s = V - V_v = \frac{V}{2}$$

$$\therefore e = \frac{V_v}{V_s} = \frac{0.5V}{0.5V} = 1$$

$$2. \quad \gamma_1 = 1.8 \text{ g/cc.} \quad \text{at } w_1 = 5\% \quad , \quad e \text{ remains constant.}$$

$$\gamma_2 = ? \quad \text{at } w_2 = 10\%$$

$$\gamma_d = \frac{\gamma}{1+w} \Rightarrow \gamma = \gamma_d (1+w) = \frac{\gamma_w G}{1+e} (1+w)$$

$$\therefore \gamma \propto (1+w) \quad (e \text{ is constant})$$

$$\frac{1.8}{\gamma_2} = \frac{1.05}{1.1}$$

$$\underline{\underline{\gamma_2 = 1.886 \text{ g/cc}}}$$

8 (7)

4. Volume of soil = vol. of sampler.

$$V = 45 \text{ cc.}$$

Given  $V_s = 25 \text{ cc.}$ 

$$e = \frac{V - V_s}{V_s} = \frac{20}{25} = \underline{0.8}$$

5. Initial wt. of soil =  $0.18 \text{ kg.}$ Water added =  $0.02.$  $\therefore$  Total weight  $w = 0.2 \text{ kg.}$ Vol. of soil,  $V = 10^{-4} \text{ m}^3$  (initial volume assumed to be constant)

$$\gamma = \frac{w}{V} = \frac{0.2}{10^{-4}} = 2000 \text{ kg/m}^3$$

$$\gamma_d = 1600 \text{ kg/m}^3$$

$$\gamma_d = \frac{\gamma}{1+w}$$

$$1600 = \frac{2000}{1+w}$$

$$w = 0.25 = \underline{25\%}$$

(method is valid only if  
V remains const. after adding  
water)

(OR)

Initial weight,  $w = 0.18 \text{ kg.}$ ,  $V = 10^{-4} \text{ m}^3$ ,  $\gamma_d = 1600 \text{ kg/m}^3$ 

$$\gamma_d = \frac{w_s}{V}$$

$$w = w_s + w_w$$

$$\therefore w_w = 0.02 \text{ kg.}$$

$$1600 = \frac{w_s}{10^{-4}} \Rightarrow w_s = 0.16 \text{ kg.} \quad (\text{water present ini})$$

Water added additionally,  $= 0.02 \text{ kg.}$ 

$$w_w = 0.02 + 0.02 = 0.04 \text{ kg.}$$

$$\text{Final water content} = \frac{0.04}{0.16} \times 100 = \underline{25\%}$$

6.  $w = 34.62 \text{ g}$ ,  $V = 24.66 \text{ cm}^3$ ,  $w_d = w_s = 20.36 \text{ g}$ 

$$G = 2.68 \quad ; \quad e = ? \quad S_r = ?$$

$$\omega = \frac{w - w_d}{w_d} \times 100 = 70\%$$

$$e = \frac{\omega G}{S_r} = \frac{0.7 \times 2.68}{S_r} \quad \rightarrow ①$$

$$\gamma = \frac{w}{V} = \frac{34.62}{24.66} = 1.40 \text{ g/cc.}$$

$$\gamma = \gamma_w \left( \frac{G_r + e S_r}{1+e} \right) \Rightarrow 1.4 = \frac{1 (2.68 + 0.7 \times 2.68)}{1+e}$$

$$\therefore e = 2.25$$

$$S_r = \frac{0.7 \times 2.68}{2.25} = \underline{\underline{83.4\%}}$$

(OR)

$$\gamma_d = \frac{w_d}{V} = \frac{20.36}{24.66} = 0.825 \text{ g/cc}$$

$$\gamma_d = \frac{G \gamma_w}{1+e}$$

$$e = \underline{\underline{2.25}}$$

$$Q19. \omega = 18\%, \gamma = 2.05 \text{ g/cc.}, G_r = 2.67,$$

$$\gamma_d = \frac{G \gamma_w}{1+e} = \frac{\gamma}{1+\omega}$$

$$\frac{2.67 \times 1}{1+e} = \frac{2.05}{1.18}$$

$$e = \underline{\underline{0.54}}$$

$$e = \frac{\omega G_r}{S_r} \Rightarrow S_r = \frac{0.18 \times 2.67}{0.54} = \underline{\underline{89.52\%}}$$

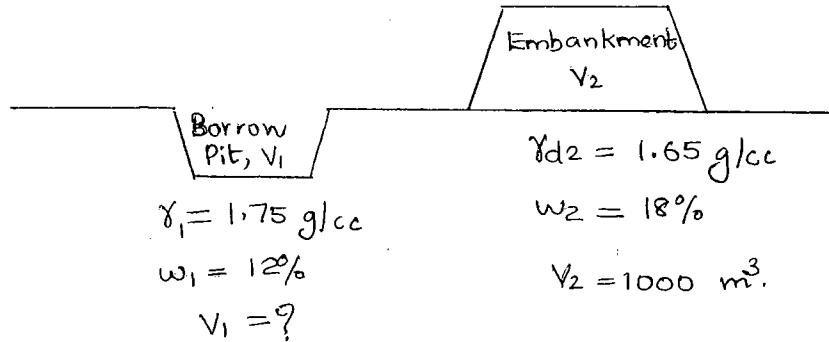
$$8. \omega = 39.3\%, G_m = \frac{\gamma_{sat}}{\gamma_w} = 1.84 \quad (\text{Soil is saturated})$$

$$e = \frac{\omega G}{S_r} = 0.393 G$$

$$\gamma_{sat} = \frac{\gamma_w (G+e)}{1+e}$$

$$1.84 = \frac{G + 0.393 G}{1 + 0.393 G} \Rightarrow G = 2.70, e = 0.393 G = \underline{\underline{1.08}}$$

10.



$$\gamma_{d1} = \frac{\gamma_1}{1+w_1} = \frac{1.75}{1+0.12} = 1.57$$

$$\frac{V_1}{V_2} = \frac{\gamma_{d2}}{\gamma_{d1}} \rightarrow 1 \text{ g/cc} = 1 \text{ ton/m}^3$$

$$V_1 = \frac{1.65}{1.57} \times 1000 = \underline{\underline{1056 \text{ m}^3}}$$

To raise w.c from  $w_1 \rightarrow w_2$ :

Weight of water to be added =  $\gamma_d V (w_2 - w_1)$

$$= 1.65 \times 1000 \times 1000 \frac{(0.18 - 0.12)}{\text{g/cc}} = 99000 \text{ kg} = \underline{\underline{99 \text{ tons}}}$$

$$\begin{aligned} \gamma_{\text{borrow}} &= 1.66 & \gamma_{\text{truck}} &= 1.15 & \gamma_{\text{fu}} &= 1.82 \\ w_b &= 8 & w_t &= 6. & w_f &= 14. \\ V_b &= ? & V_t &= ? & V_f &= 100 \text{ m}^3 \\ \gamma_{d1} &= 1.54 & \gamma_{d2} &= 1.08 & \gamma_{d3} &= 1.25. \end{aligned}$$

$$\frac{V_2}{V_3} = \frac{\gamma_{d3}}{\gamma_{d2}}$$

$$V_2 = \frac{1.25}{1.08} \times 100 = 147 \text{ m}^3$$

$$\text{No: of truck loads} = \frac{147}{6} = 24.6 \text{ no.s} = \underline{\underline{25 \text{ no.s}}}$$

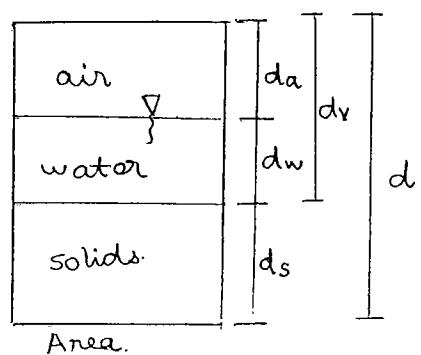
13.

If  $d_w = 1 \text{ m}$ ,  $d = ?$

Given:  $c = 0.5$ ,  $S_r = 80\%$

$$S_r = \frac{d_w}{d_v}$$

$$d_v = \frac{1}{0.8} = 1.25 \text{ m}$$



depth of voids,  $d_v = 1.25 \text{ m}$ .

$$e = \frac{d_v}{d_s} \Rightarrow d_s = \frac{1.25}{0.5} = \underline{\underline{2.5 \text{ m}}}$$

Total depth of soil,  $d = d_s + d_v = \underline{\underline{3.75 \text{ m}}}$

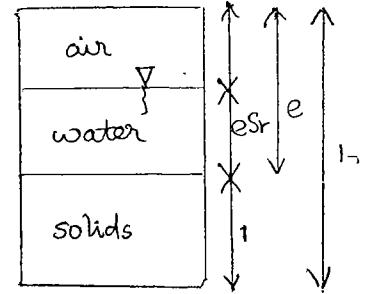
(OR) based on Unit Phase Diagram.

Given, depth of water = 1 m

$$e_{Sr} = 0.5 \times 0.8 = 0.4 \text{ m}$$

$$1+e = 1+0.5 = 1.5 \text{ m}$$

0.4 m depth of water makes 1.5 m depth of soil 80% saturated.



$$\text{Depth of soil} = \frac{1.5}{0.4} \times 1 = \underline{\underline{3.75 \text{ m}}}$$

14.  $n = 40\%, G = 2.5, \omega = 12\%$

$$n = \frac{e}{1+e}$$

$$e = \frac{n}{1-n} = \frac{0.4}{1-0.4} = 0.666.$$

$$e = \frac{\omega G}{Sr} = \omega G \text{ (at full saturation).}$$

$$\omega = \frac{2}{3 \times 2.5} = 26.6\%$$

15.  $\gamma_d = \frac{\gamma_w G}{1+e}$

$$\text{Take } \gamma_w = 1 \text{ ton/m}^3. \Rightarrow \gamma_d = 1.5 \text{ t/m}^3.$$

Weight of water to be added to achieve full saturation  
 $= 1.5 \times 100 \left( \frac{26.6 - 12}{100} \right)$   
 $= \underline{\underline{21.9 \text{ tons.}}}$

16. Let  $e_2$  be void ratio at increased volume of soil

$$\frac{V_2}{V_1} = \frac{1+e_2}{1+e_1}$$

$$\frac{1.05 V_1}{V_1} = \frac{1+e_2}{1+0.667} \Rightarrow e_2 = \underline{\underline{0.75}}$$

Let  $\omega_3$  be water content at increased volume,

10 (v)

$$e_2 = \frac{\omega_3 G}{S_r}$$

$$0.75 = \frac{\omega_3 \times 2.5}{S_r=1}$$

$$\Rightarrow \omega_3 = \underline{\underline{30\%}}$$

17.  $w_d = w_s = 135 \text{ g.}$

Wet weight,  $w = 195 \text{ g.}$

Total vol. of soil,  $v = 5^3 = 125 \text{ cm}^3.$

$$\omega = \frac{w - w_d}{w_d} \times 100 = \frac{60}{135} \times 100 = \underline{\underline{44.44\%}}$$

$$e = \frac{\omega G}{S_r} \Rightarrow e = 0.44 G$$

$$\gamma_{sat} = \frac{w}{v} = \frac{195}{125} = 1.56 \text{ g/cc.}$$

$$\gamma_{sat} = \gamma_w \left( \frac{G+e}{1+e} \right) \Rightarrow 1.56 = 1 \left( \frac{G + 0.44G}{1 + 0.44G} \right) \\ = 2.07.$$

$$e = 0.44 G = \underline{\underline{0.92}}$$

(OR)

$$v = 125 \text{ cm}^3.$$

wt. of water added =  $195 - 135 = 60 \text{ g}$

$$\therefore \text{Vol. of water} = \frac{w_w}{\gamma_w} = \frac{60}{1} = \underline{\underline{60 \text{ g/cc}}}$$

Vol. of voids = vol. of water added

$$\Rightarrow V_v = 60 \text{ cc. } \therefore V_s = 125 - 60 = 65 \text{ cm}^3.$$

$$e = \frac{V_v}{V_s} = 0.92.$$

$$\gamma_s = \frac{w_s}{V_s} = \frac{135}{65} = 2.07 \text{ g/cc} \Rightarrow G_i = \frac{\gamma_s}{\gamma_w} = \frac{2.07}{1} = \underline{\underline{2.07}}$$

Q. 100 g of dry soil having  $G = 2.7$  is mixed with water and 1 L of soil slurry is prepared. What is the unit weight of soil slurry in g/cc.

$$G = \frac{\gamma_s}{\gamma_w} \Rightarrow \gamma_s = 2.7 \text{ g/cc}$$

$$\gamma_s = \frac{w_s}{v_s} \therefore v_s = \frac{w_s}{\gamma_s} = \frac{100}{2.7} = 37.037 \text{ cc}$$

$$V = 1 \text{ L} = 1000 \text{ cm}^3 \Rightarrow V_w = V - v_s = 1000 - 37.037$$

$$\begin{aligned} \gamma_{sat} &= \frac{w}{v} = \frac{w_s + w_w}{v} = \frac{100 + V_w \times \gamma_w}{v} \\ &= \frac{100 + 962.963 \times 1}{1000} = \underline{1.063} \text{ g/cc} \end{aligned}$$

16th Aug,  
SATURDAY  
Q A marine soil has sp.gr. of solids as 2.7 and void ratio as

0.8. If sp.gr. of sea water is 1.024, calculate  $\gamma_{sat}$  of the soil. Take  $\gamma_w$  of fresh water as  $9.81 \text{ kN/m}^3$ .

$$G = 2.7$$

$$\Rightarrow \gamma_{soil} \gamma_s = 2.7 \text{ g/cc.}$$

$$e = 0.8.$$

$$\gamma_{sat} = \frac{\gamma_w (G+e)}{1+e} = \frac{19.64}{1.8} \quad (\text{valid only if } \text{sea water is used!})$$

$$G = \frac{\gamma_s}{\gamma_w} \rightarrow \text{pure water}$$

$$\begin{aligned} \gamma_{sat} &= \frac{w_w + w_s}{v} = \frac{v_s \gamma_s + V_w \gamma_{sea \text{ water}}}{v} \\ &= \frac{1 \times G \gamma_w + e \cdot Sr \times 10.1043}{1+e} \\ &= \frac{9.81 \times 2.7 \times 1 + 0.8 \times 1 \times 10.1043}{1+0.8} \\ &= \underline{\underline{19.206}} \text{ kN/m}^3 \end{aligned}$$

$$\begin{aligned} \gamma_{sea} &= 1.03 \times 9.81 \\ &= 10.1043 \text{ g/cc} \end{aligned}$$

$$\left. \begin{aligned} v_s &= 1 \\ v_w &= e \cdot Sr \\ v &= 1+e \end{aligned} \right\} \begin{array}{l} \text{unit} \\ \text{phase} \end{array}$$

Q The mass of an empty pycnometer is 0.498 kg. When completely filled with water its mass is found to be 1.528 kg. An oven dried soil of mass 0.198 kg is placed in the pycnometer and water is added to fill the pycnometer and the total mass is found to be 1.653 kg. Determine sp. gravity of soil particles.

$$W_1 = 0.498, \quad W_2 = 0.198, \quad W_3 = 1.653, \quad W_4 = 1.528.$$

$$G = \frac{W_2 - W_1}{(W_2 - W_1) - (W_3 - W_4)} = \frac{0.198}{0.198 - 0.125} = \underline{\underline{2.712}}$$

Wt. of dry soil  
=  $W_2 - W_1$   
= 0.198

Q A sample of clay was coated with paraffin wax and the total mass of soil and wax was found to be 700 g. The sample was immersed in water and the vol. of water displaced was found to be 355 mL. The mass of the sample without wax was 690 g. and water content of the soil was 18%. Determine bulk density, dry density, void ratio and degree of saturation. Take sp.gr. of soil solids as 2.7 and that of wax as 0.89.

$$355 \times 1 = \frac{690}{2.7} + \frac{10}{0.89}$$

$$\text{Weight of wax} = 700 - 690 = 10 \text{ g.}$$

$$\text{Density of wax} = 0.89 \times 1 = 0.89 \text{ g/cc.}$$

$$\text{Volume of wax} = \frac{10}{0.89} = 11.236 \text{ cc.}$$

$$\begin{aligned} \text{Volume of soil} &= \text{vol. of water displaced} - \text{vol. of wax} \\ &= 355 - 11.23 = \underline{\underline{343.77}} \text{ cm}^3 \end{aligned}$$

$$\gamma = \frac{w}{v} = \frac{690}{343.77} = 2.007 \text{ g/cc.}$$

$$\gamma_d = \frac{\gamma}{1+w} = \frac{2.007}{1.18} = 1.7 \text{ g/cc}$$

$$\gamma_d = \frac{G \gamma_w}{1+e} \Rightarrow e = \underline{\underline{0.588}}$$

$$e = \frac{wG}{S_r} \Rightarrow S_r = \underline{\underline{82.65\%}}$$