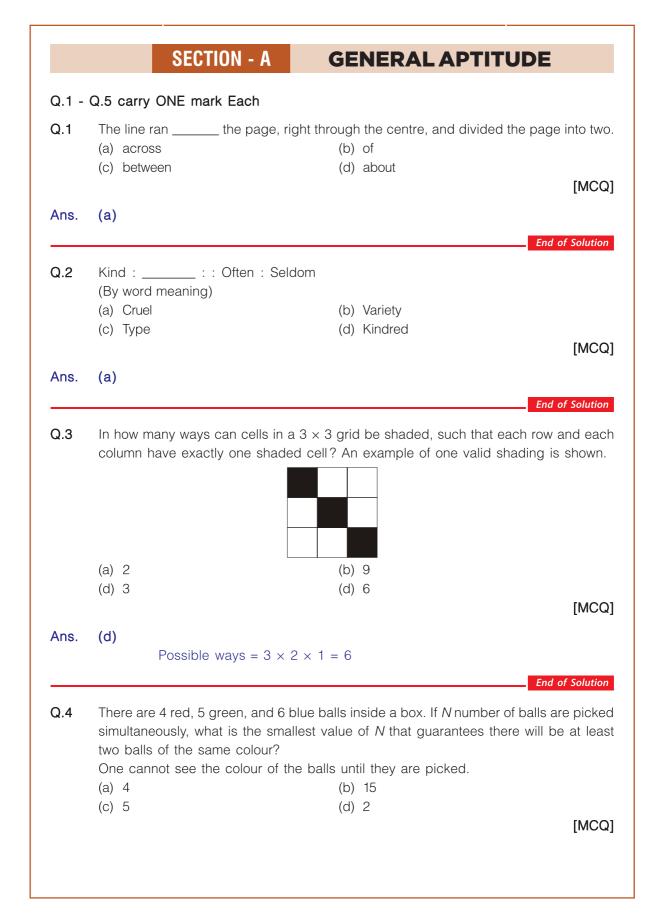
# GATE 2023 Civil Engineering Exam Held on : 12-2-2023 Afternoon Session





Given: 4 Red, 5 Green and 6 Blue We select three balls in worst case 1 Red, 1 Green and 1 Blue If we select fourth ball then we found two balls are of same colour.

End of Solution

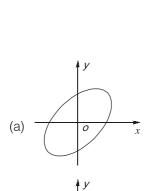
Q.5 Consider a circle with its centre at the origin (O), as shown. Two operations are allowed on the circle.

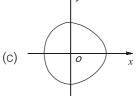
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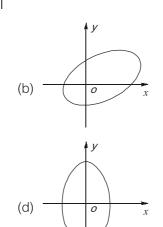
Operation 1: Scale independently along the x and y axes.

Operation 2: Rotation in any direction about the origin.

Which figure among the options can be achieved through a combination of these two operations on the given circle?



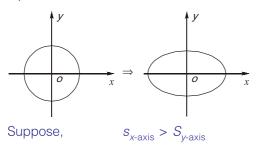


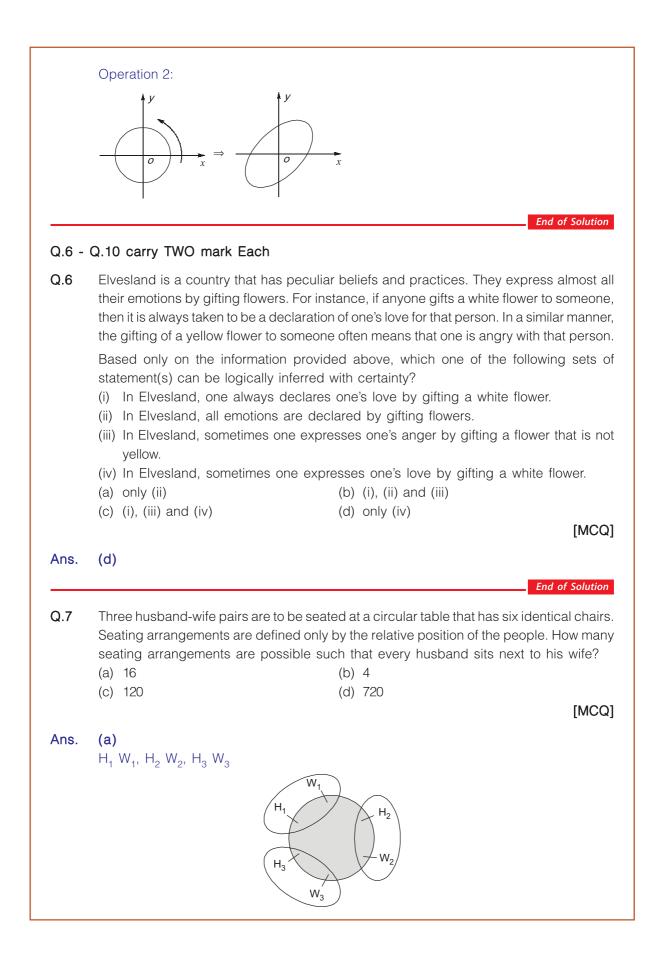


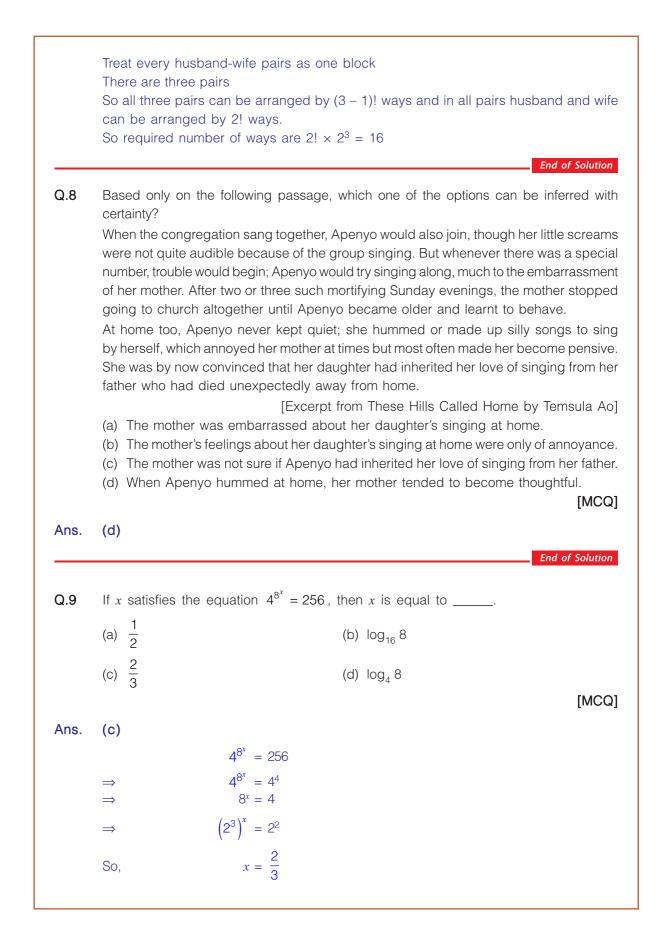
[MCQ]

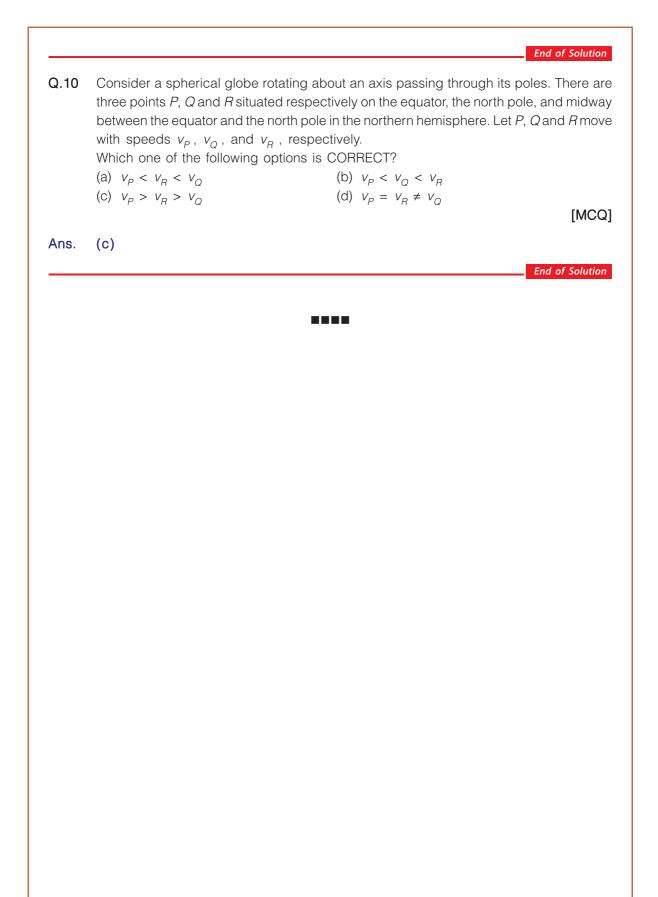
Ans.

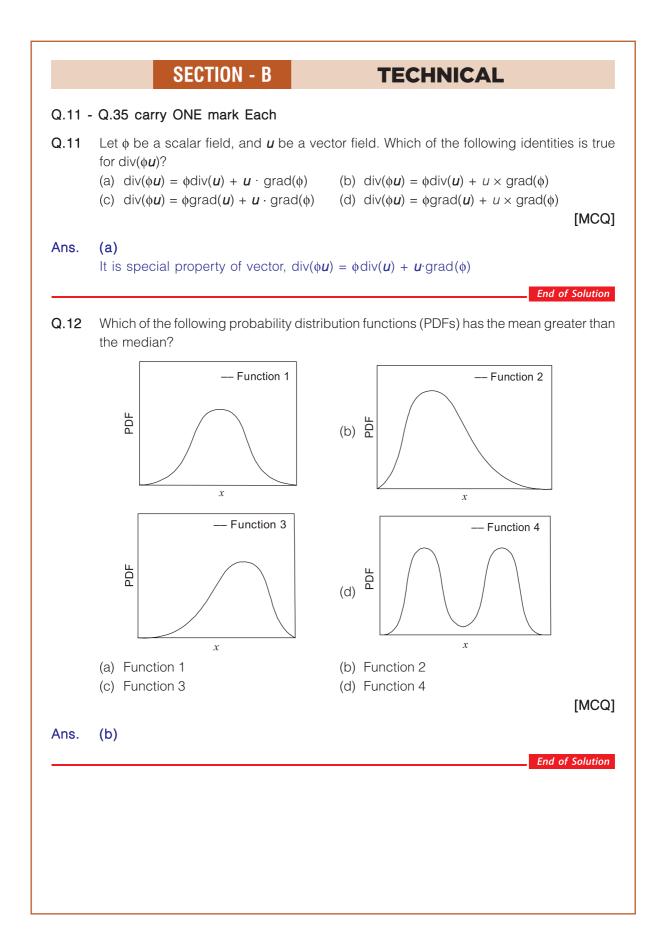




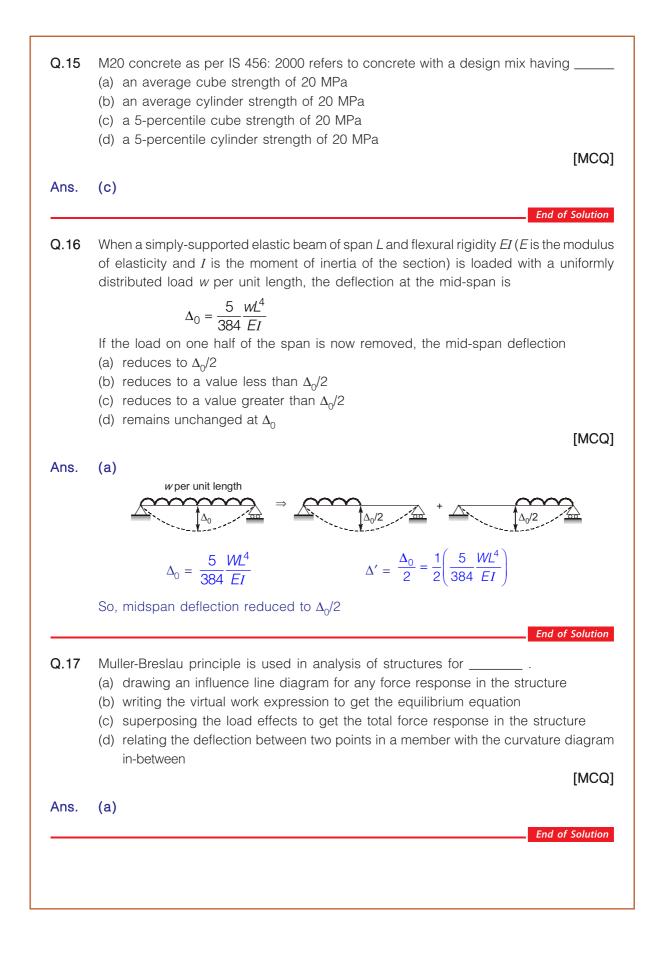








Q.13	A remote village has exactly 1000 vehicles with sequential registration numbers starting from 1000. Out of the total vehicles, 30% are without pollution clearance certificate. Further, even- and odd-numbered vehicles are operated on even- and odd-numbered dates, respectively.
	If 100 vehicles are chosen at random on an even-numbered date, the number of vehicles expected without pollution clearance certificate is (a) 15 (b) 30
	(c) 50 (d) 70 [MCQ]
Ans.	<ul> <li>(b)</li> <li>Probability of selecting even vehicles on even numbered dates = 1</li> <li>∴ The no. of vehicles expected without pollution clearance certificate</li> <li>= 100 × 1 × 0.3</li> <li>= 30</li> </ul> End of Solution
Q.14	A circular solid shaft of span $L = 5$ m is fixed at one end and free at the other end. A torque $T = 100$ kN.m is applied at the free end. The shear modulus and polar moment of inertia of the section are denoted as $G$ and $J$ , respectively. The torsional rigidity $GJ$ is 50,000 kN.m <sup>2</sup> /rad. The following are reported for this shaft: Statement i) The rotation at the free end is 0.01 rad Statement ii) The torsional strain energy is 1.0 kN.m With reference to the above statements, which of the following is true? (a) Both the statements are correct (b) Statement i) is correct, but Statement ii) is wrong (c) Statement i) is wrong, but Statement ii) is correct (d) Both the statements are wrong
<b>A</b> = 0	[MCQ]
Ans.	(b) Statement (i):
	Rotation at free end ( $\theta$ ) = $\frac{\tau L}{GJ} = \frac{100 \times 5 (\text{kNm}^2)}{50000 (\text{kNm}^2/\text{rad})}$
	= 0.01 rad Statement (ii):
	Torsional strain energy = $\frac{1}{2} \times T \times \theta = \frac{1}{2} \times 100 \times 0.01$
	= 0.5 kN-m



Q.18 A standard penetration test (SPT) was carried out at a location by using a manually operated hammer dropping system with 50% efficiency. The recorded SPT value at a particular depth is 28. If an automatic hammer dropping system with 70% efficiency is used at the same location, the recorded SPT value will be

 (a) 28
 (b) 20

(d) 25

## Ans. (b)

(c) 40

(0)	
	Efficiency $\propto \frac{1}{\text{Number of blows}} \propto \frac{1}{SPT \text{ value}}$
$\Rightarrow$	$\eta_1 N_1 = \eta_2 N_2$
$\Rightarrow$	$0.5 \times 28 = 0.7 \times N_2$
$\Rightarrow$	$N_2 = \frac{0.5 \times 28}{0.7} = 20$

End of Solution

[MCQ]

**Q.19** A vertical sheet pile wall is installed in an anisotropic soil having coefficient of horizontal permeability,  $k_H$  and coefficient of vertical permeability,  $k_V$ . In order to draw the flow net for the isotropic condition, the embedment depth of the wall should be scaled by a factor of \_\_\_\_\_, without changing the horizontal scale.

(a)	$\sqrt{\frac{k_H}{k_V}}$	(b)	$\sqrt{\frac{k_V}{k_H}}$
(C)	1.0	(d)	$\frac{k_H}{k_V}$

[MCQ]

## Ans. (a)

A steady-state, homogeneous, anisotropic sysem can be mathematically transformed into an isotropic system by coordinate transformation, creating what is sometimes called a transformed section.

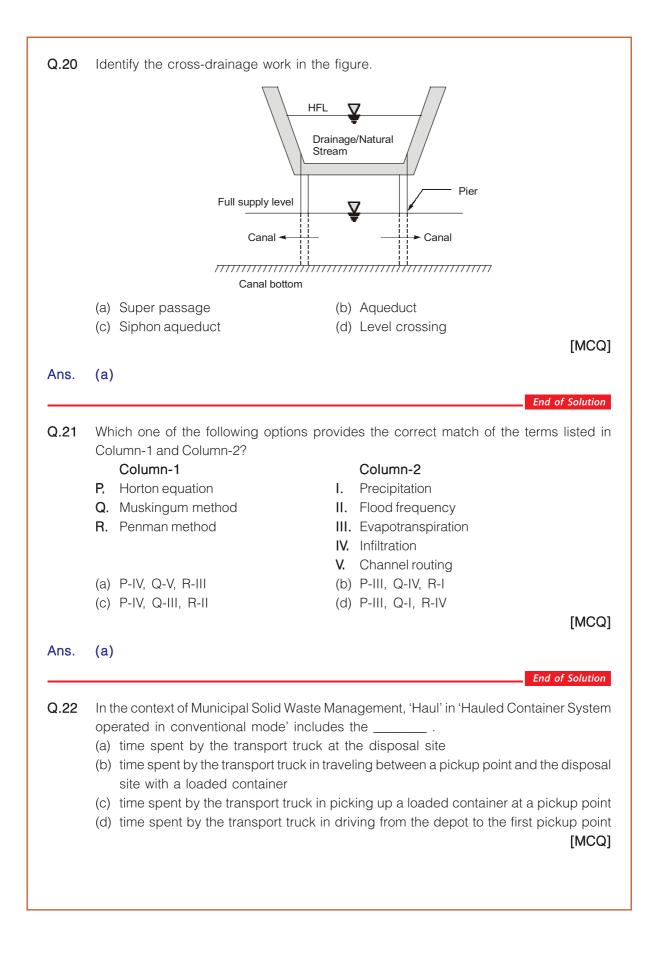
The coordinates in the true anisotropic sysem are x and z. In the tranformed isotropic system the coordinates are X and Z, where

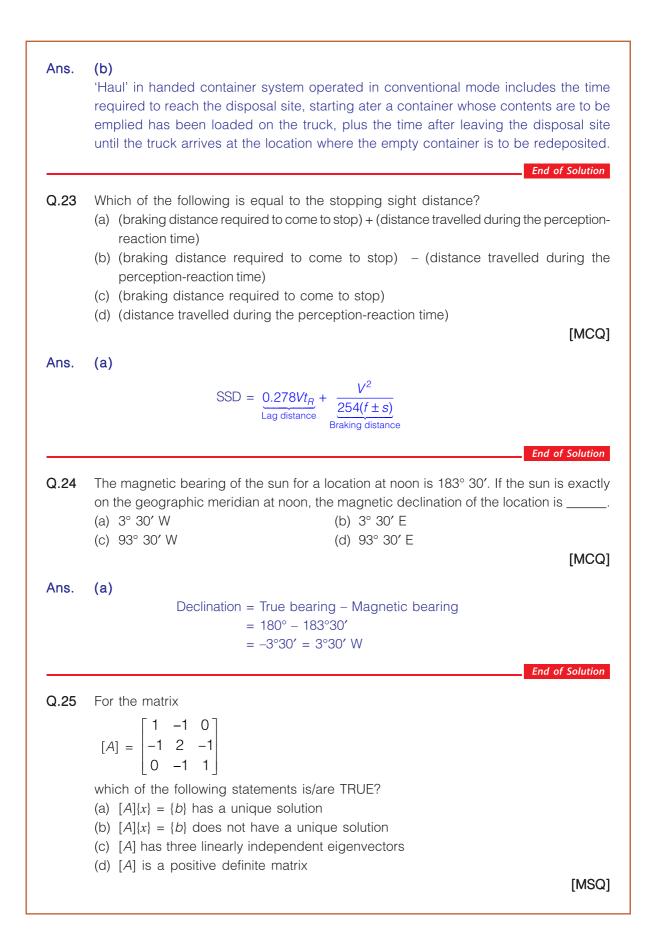
$$X = x$$
$$Z = Z \sqrt{\frac{k_x}{k_z}}$$
$$Z = Z \sqrt{\frac{k_H}{k_V}}$$

So, the embedment depth of wall should be scaled by factor of  $\sqrt{\frac{k_H}{k_V}}$ 

End of Solution

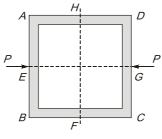
*x* 





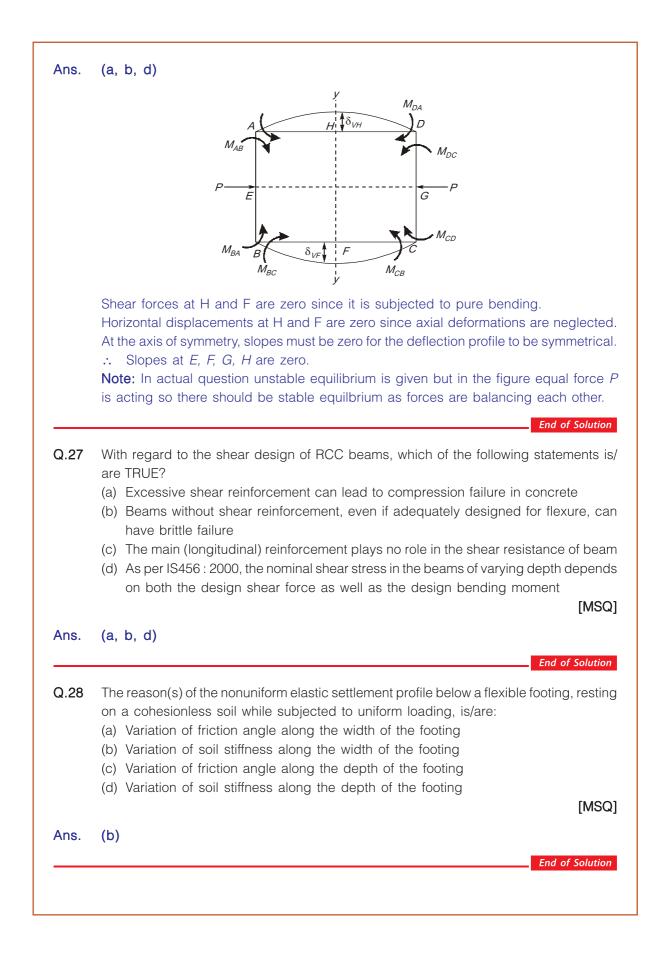
Ans. (b, c) |A| = 0As So, one of the eigen value is zero.  $|A - \lambda I| = 0$  $\begin{bmatrix} 1-\lambda & -1 & 0\\ -1 & 2-\lambda & -1\\ 0 & -1 & 1-\lambda \end{bmatrix} = 0$  $(1-\lambda)\left\lceil (2-\lambda)(1-\lambda)-1\right\rceil + 1\left\lceil \lambda-1\right\rceil = 0$  $(1 - \lambda)[(2 - \lambda)(1 - \lambda) - 1 - 1] = 0$  $\lambda(1-\lambda)(3-\lambda)=0$  $\lambda = 0, 1, 3$ As there are three eigen values so number of linearly independent eigen vector are 3. |A| = 0Since, So,  $[A]{x} = {b}$  does not have a unique solution For the positive definite matrix, all the eigen values must be positive. But, here one eigen value i.e.  $\lambda = 0$ , so, A is not a positive definite matrix. End of Solution

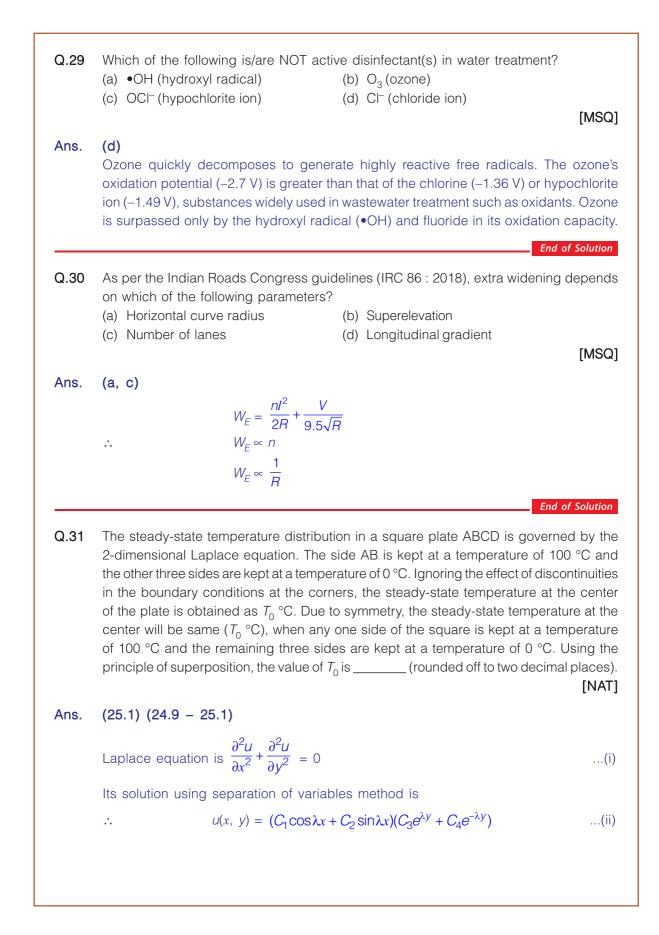
**Q.26** In the frame shown in the figure (not to scale), all four members (AB, BC, CD, and AD) have the same length and same constant flexural rigidity. All the joints A, B, C, and D are rigid joints. The midpoints of AB, BC, CD, and AD, are denoted by E, F, G, and H, respectively. The frame is in unstable equilibrium under the shown forces of magnitude *P* acting at E and G. Which of the following statements is/are TRUE?



- (a) Shear forces at H and F are zero
- (b) Horizontal displacements at H and F are zero
- (c) Vertical displacements at H and F are zero
- (d) Slopes at E, F, G, and H are zero

[MSQ]





AB = CD = 1Let u(x, 1) = 100Using u(x, 0) = 0u(0, y) = 0С  $C_{3} = -C_{4}$  $\Rightarrow$ *u*(1, *J*) = 0 Using (0, y) = 0 $C_1 = 0$  $\Rightarrow$ Substituting constants values in equation (ii)  $u(x, y) = (0 + C_2 \sin\lambda x) \left( C_3 e^{\lambda y} - C_3 e^{-\lambda y} \right)^{-1} \frac{1}{4} u(x, 0) = 0$ В  $u(x, y) = a_n \sin \lambda x (e^{\lambda y} - e^{-\lambda y})$ Now, using u(1, y) = 0,  $\sin \lambda = 0 \Rightarrow \lambda = n\pi$  $u(x, y) = \sum a_n \sin(n\pi x) \Big[ e^{n\pi y} - e^{-n\pi y} \Big]$ Hence ...(iii) u(x, 1) = 100Using  $a_n = \frac{100}{\sin n\pi x \left[ e^{n\pi} - e^{-n\pi} \right]}$  $\Rightarrow$ Using equation (iii),

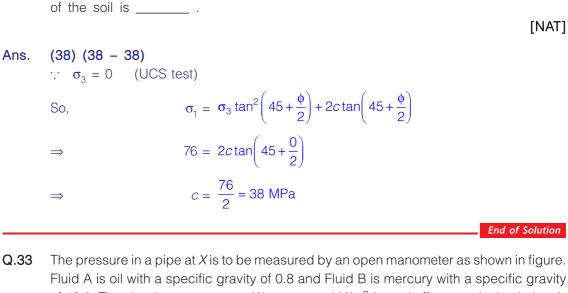
$$U(x, y) = \sum_{n=1}^{\infty} \frac{100}{e^{n\pi} - e^{-n\pi}} \left( e^{n\pi y} - e^{-n\pi y} \right)$$

At mid point,

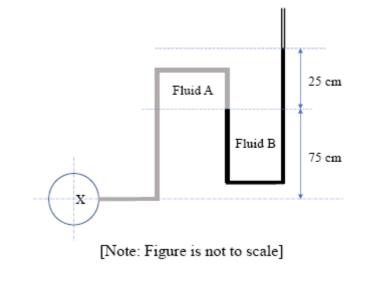
So,

$$\begin{aligned} \mathcal{U}\left(\frac{1}{2},\frac{1}{2}\right) &= T_{0} \\ T_{0} &= \sum_{n=1}^{\infty} \frac{100}{\left(e^{n\pi} - e^{-n\pi}\right)} \left(e^{\frac{n\pi}{2}} - e^{\frac{-n\pi}{2}}\right) \\ &= 100 \left[\left(\frac{e^{\pi}}{e^{2}} - e^{\frac{-\pi}{2}}\right) + \left(\frac{e^{\pi}}{e^{2\pi}} - e^{-\pi}\right) + \frac{e^{\pi}}{e^{2\pi}} - e^{-\pi}\right) + \frac{e^{\pi}}{e^{2\pi}} + \frac{1}{e^{\pi}} + \frac{1}{e^{\pi}} + \frac{1}{e^{2\pi}} + \frac{1}{e^{2\pi}}$$

**Q.32** An unconfined compression strength test was conducted on a cohesive soil. The test specimen failed at an axial stress of 76 kPa. The undrained cohesion (in kPa, *in integer*) of the soil is \_\_\_\_\_\_.



Fluid A is oil with a specific gravity of 0.8 and Fluid B is mercury with a specific gravity of 13.6. The absolute pressure at X is \_\_\_\_\_\_ kN/m<sup>2</sup> (round off to one decimal place). [Assume density of water as 1000 kg/m<sup>3</sup> and acceleration due to gravity as 9.81 m/s<sup>2</sup> and atmospheric pressure as 101.3 kN/m<sup>2</sup>]



[NAT]

Ans. (140.5) (140 - 141)  $P_x - (800 \times 9.81 \times 0.75) - (13600 \times 9.81 \times 0.25) = P_{\text{atm}}$   $P_x = (101.3 \times 10^3) + (800 \times 9.81 \times 0.75) + (13600 \times 9.81 \times 0.25)$   $P_x = 140540 \text{ N/m}^2$  $P_x = 140.54 \text{ kN/m}^2$ 

**Q.34** For the elevation and temperature data given in the table, the existing lapse rate in the environment is \_\_\_\_\_\_ °C/100 m (round off to two decimal places).

Elevation from ground level (m)	Temperature (°C)
5	14.2
325	16.9

[NAT]

## Ans. (0.84) (0.84 - 0.85)

The existing lapse rate in the environment =  $-\frac{\Delta T}{\Delta H} = -\frac{16.9 - 14.2}{3.25 - 5}$  °C/m

$$= 2.7 \times \frac{100}{320} \circ C/100 \text{ m}$$
$$= 0.84 \circ C/100 \text{ m}$$

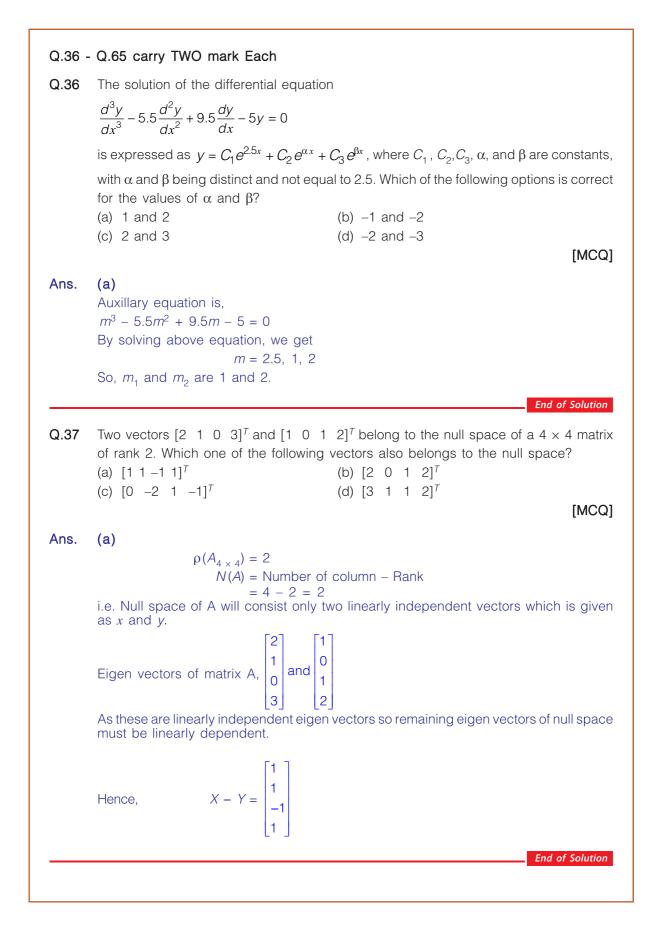
End of Solution

**Q.35** If the size of the ground area is 6 km × 3 km and the corresponding photo size in the aerial photograph is 30 cm × 15 cm, then the scale of the photograph is 1 : \_\_\_\_\_ (in integer)

[NAT]

#### Ans. (20000) (20000 - 20000)

Scale = 
$$\frac{\text{Photodistance}}{\text{Ground distance}} = \frac{30 \text{ cm}}{6 \text{ km}} = \frac{30 \text{ cm}}{600000 \text{ cm}} = \frac{1}{20000}$$
  
Alternatively,  $(\text{Scale})^2 = \left(\frac{\text{Area of map}}{\text{Area on ground}}\right)$   
 $= \frac{(30 \times 15)}{6 \times 3} = \frac{30 \times 15}{(6 \times 3) \times (100000)^2}$   
 $\Rightarrow \qquad \text{Scale} = \sqrt{\frac{25}{(100000)^2}}$   
 $\therefore \qquad S = \frac{1}{20000}$   
*End of Solution*



**Q.38** Cholesky decomposition is carried out on the following square matrix [*A*].

$$[A] = \begin{bmatrix} 8 & -5 \\ -5 & a_{22} \end{bmatrix}$$

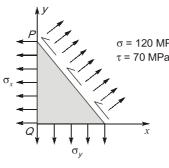
Let  $I_{ij}$  and  $a_{ij}$  be the (i, j)<sup>th</sup> elements of matrices [L] and [A], respectively. If the element  $I_{22}$  of the decomposed lower triangular matrix [L] is 1.968, what is the value (rounded off to the nearest integer) of the element  $a_{22}$ ? (a) 5 (b) 7

(a) 5 (b) 7 (c) 9 (d) 11

Ans. (b)

 $\mathcal{L}\mathcal{L}^{T} = A$   $\begin{bmatrix} L_{11} & 0\\ L_{21} & L_{22} \end{bmatrix} \begin{bmatrix} L_{11} & L_{21}\\ 0 & L_{22} \end{bmatrix} = \begin{bmatrix} 8 & -5\\ -5 & a_{22} \end{bmatrix}$   $L_{11} = 2\sqrt{2}, \ L_{21} = -\frac{5}{2\sqrt{2}} \text{ and } L_{21}^{2} + L_{22}^{2} = a_{22}$   $a_{22} = \left(-\frac{5}{2\sqrt{2}}\right)^{2} + 1.968 = 6.99 \simeq 7$ 

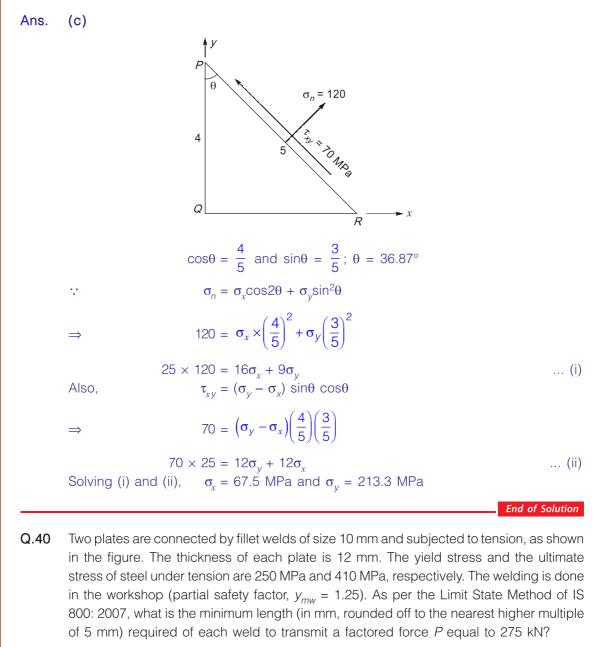
**Q.39** In a two-dimensional stress analysis, the state of stress at a point is shown in the figure. The values of length of *PQ*, *QR*, and *RP* are 4, 3, and 5 units, respectively. The principal stresses are \_\_\_\_\_\_\_\_\_. (round off to one decimal place)

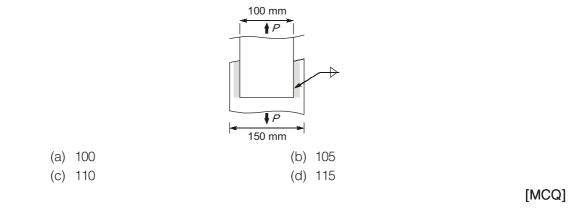


(a)  $\sigma_x = 26.7$  MPa,  $\sigma_y = 172.5$  MPa (b)  $\sigma_x = 54.0$  MPa,  $\sigma_y = 128.5$  MPa (c)  $\sigma_x = 67.5$  MPa,  $\sigma_y = 213.3$  MPa (d)  $\sigma_x = 16.0$  MPa,  $\sigma_y = 138.5$  MPa

[MCQ]

[MCQ]





Ans. (b)

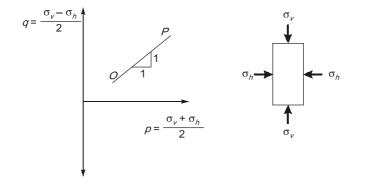
$$P = L_w(ks) \frac{f_u}{\sqrt{3}\gamma_{mw}}$$

$$275 \times 10^3 = L_w \times 0.7 \times 10 \times \frac{410}{\sqrt{3} \times 1.25}$$

$$L_w = 207.45 \text{ mm}$$
Length of each weld =  $\frac{207.45}{2} = 103.73 \text{ mm} \simeq 105 \text{ mm}$ 

End of Solution

**Q.41** In the given figure, Point *O* indicates the stress point of a soil element at initial nonhydrostatic stress condition. For the stress path (OP), which of the following loading conditions is correct?



(a)  $\sigma_v$  is increasing and  $\sigma_h$  is constant

(b)  $\sigma_v$  is constant and  $\sigma_h$  is increasing

(c)  $\sigma_{\!_{\scriptscriptstyle V}}$  is increasing and  $\sigma_{\!_h}$  is decreasing

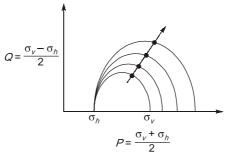
(d)  $\sigma_{\!_{\scriptscriptstyle V}}$  is decreasing and  $\sigma_{\!_{\scriptscriptstyle h}}$  is increasing

[MCQ]



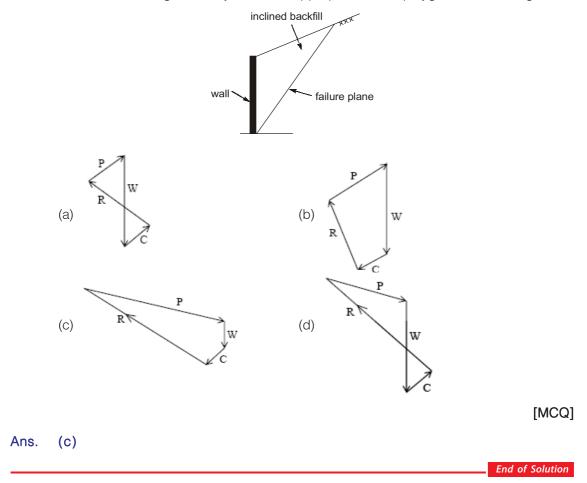
 $\sigma_v$  = Major principal stress

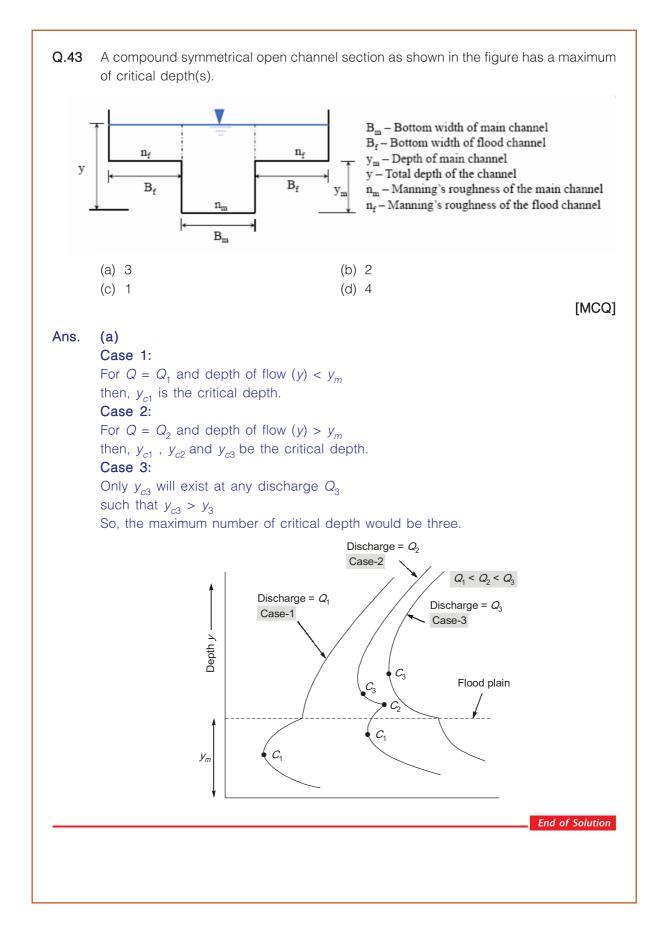
 $\sigma_h$  = Minor principal stress



So,  $\sigma_h$  is constant and  $\sigma_v$  is increasing.

**Q.42** The figure shows a vertical retaining wall with backfill consisting of cohesive-frictional soil and a failure plane developed due to passive earth pressure. The forces acting on the failure wedge are: P as the reaction force between the wall and the soil, R as the reaction force on the failure plane, C as the cohesive force along the failure plane and W as the weight of the failure wedge. Assuming that there is no adhesion between the wall and the wedge, identify the most appropriate force polygon for the wedge.





## Q.44 The critical flow condition in a channel is given by \_\_\_\_\_. [Note: $\alpha$ – kinetic energy correction factor; Q – discharge; A<sub>c</sub> – cross-sectional area of flow at critical flow condition; $T_c$ - top width of flow at critical flow condition; g - acceleration due to gravity] (a) $\frac{\alpha Q^2}{q} = \frac{A_c^3}{T_c}$ (b) $\frac{\alpha Q}{g} = \frac{A_c^3}{T_c^2}$ (c) $\frac{\alpha Q^2}{g} = \frac{A_c^3}{T_c^2}$ (d) $\frac{\alpha Q}{q} = \frac{A_c^3}{T_c}$ [MCQ] Ans. (a) Froude number, $F_r = \frac{V}{\sqrt{qD}}$ ; where D = hydraulic depth = $\frac{A}{T}$ $F_r^2 = \frac{v^2}{gD}$ $\Rightarrow$ $F_r^2 = \frac{Q^2 T}{g A^2 A} = \frac{Q^2 T}{g A^3}$ $\Rightarrow$ For critical condition, $F_r = 1$

 $\frac{Q^2 T_c}{Q A^3} = 1$ 

 $\Rightarrow$ 

Now, taking into account, kinetic energy correction factor,

$$\frac{\alpha Q^2}{g} = \frac{A_c^3}{T_c}$$

 $\frac{Q^2}{q} = \frac{A_c^3}{T_c}$ 

End of Solution

Match the following air pollutants with the most appropriate adverse health effects: Q.45 Air pollutant Health effect to human and/or test animal (P) Aromatic hydrocarbons (1) Reduce the capability of the blood to carry oxygen

- (II) Bronchitis and pulmonary emphysema
- (III) Damage of chromosomes
- (S) Ozone

(Q) Carbon monoxide

(R) Sulfur oxides

- (a) (P) (II), (Q) (I), (R) (IV), (S) (III)
- (b) (P) (IV), (Q) (I), (R) (III), (S) (II)
- (c) (P) (III), (Q) (I), (R) (II), (S) (IV)
- (d) (P) (IV), (Q) (I), (R) (II), (S) (III)

Ans. (d)

- (IV) Carcinogenic effect

[MCQ]

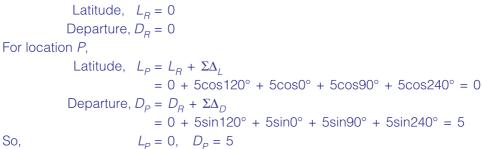
Q.46 A delivery agent is at a location R. To deliver the order, she is instructed to travel to location P along straight-line paths of RC, CA, AB and BP of 5 km each. The direction of each path is given in the table below as whole circle bearings. Assume that the latitude (L) and departure (D) of R is (0, 0) km. What is the latitude and departure of P (in km, rounded off to one decimal place)?

	Paths	RC	CA	AB	BP
	Directions (in degrees)	120	0	90	240
(a) $L = 2.5; D = 5.0$ (C) $L = 5.0; D = 2.5$		. ,			= 5.0 = 0.0

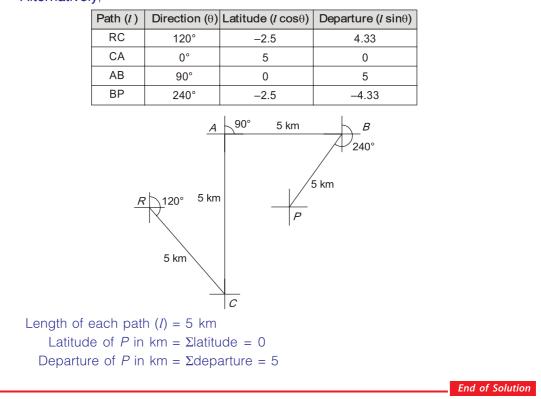
[MCQ]

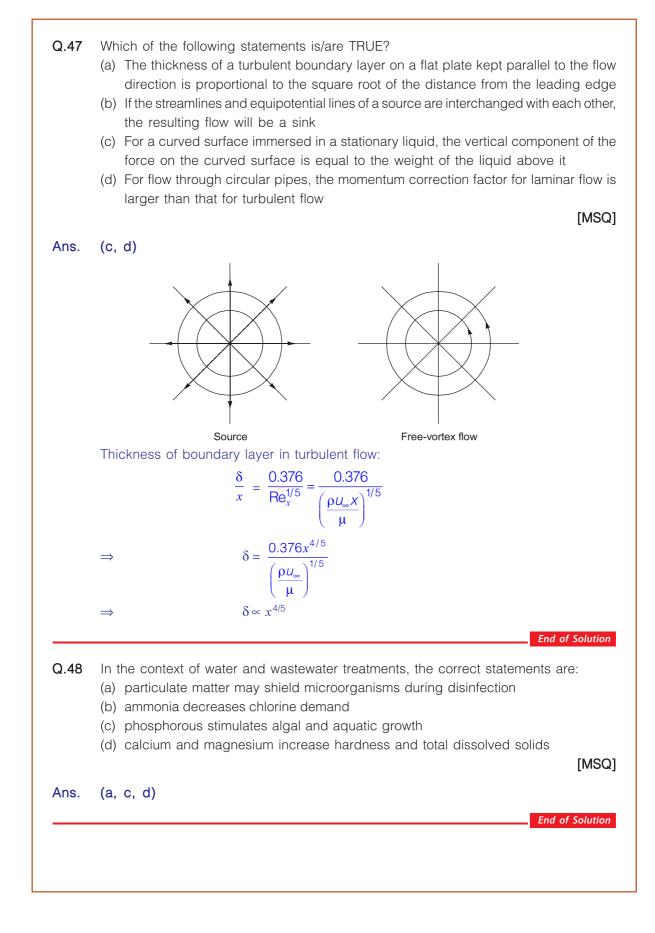
#### Ans. (b)

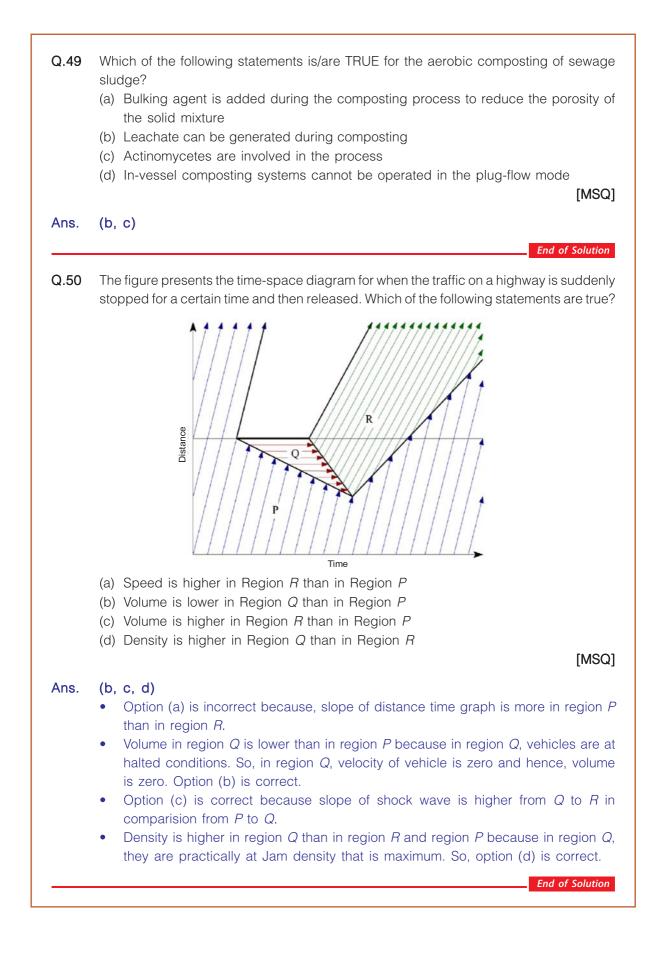
For location R,

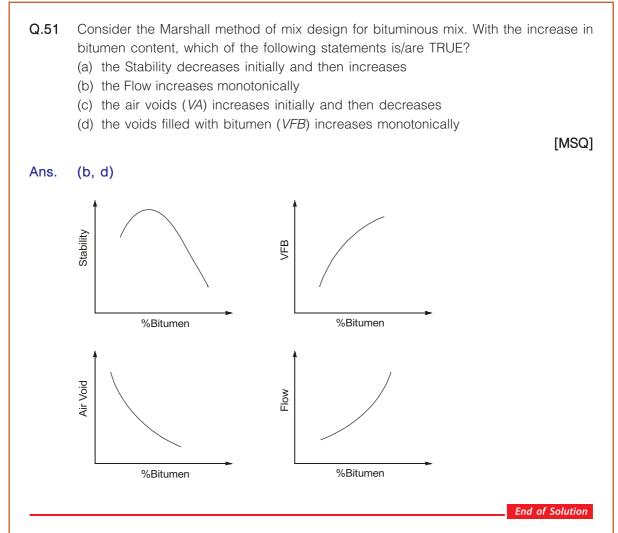


So, Alternatively,









**Q.52** A 5 cm long metal rod *AB* was initially at a uniform temperature of  $T_0$  °C. Thereafter, temperature at both the ends are maintained at 0 °C. Neglecting the heat transfer from the lateral surface of the rod, the heat transfer in the rod is governed by the one-

dimensional diffusion equation  $\frac{\partial T}{\partial t} = D \frac{\partial^2 T}{\partial x^2}$ , where D is the thermal diffusivity of the

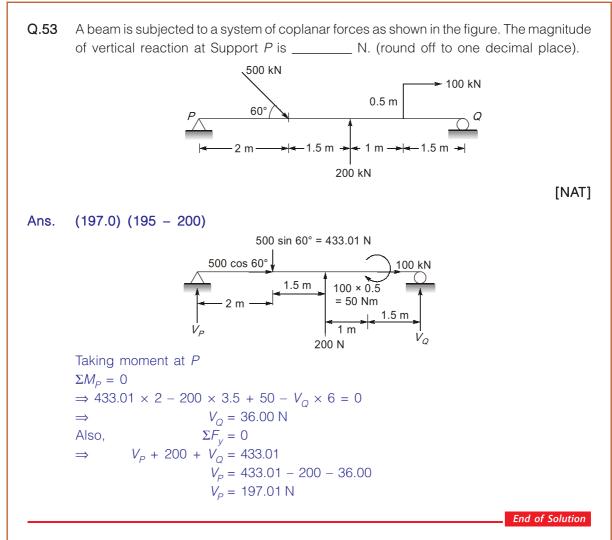
metal, given as 1.0 cm<sup>2</sup>/s.

The temperature distribution in the rod is obtained as

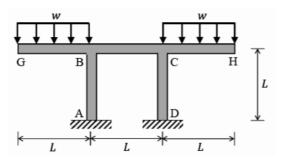
$$T(x, t) = \sum_{n=1,3,5...}^{\infty} C_n \sin \frac{n\pi x}{5} e^{-\beta n^2 t}$$

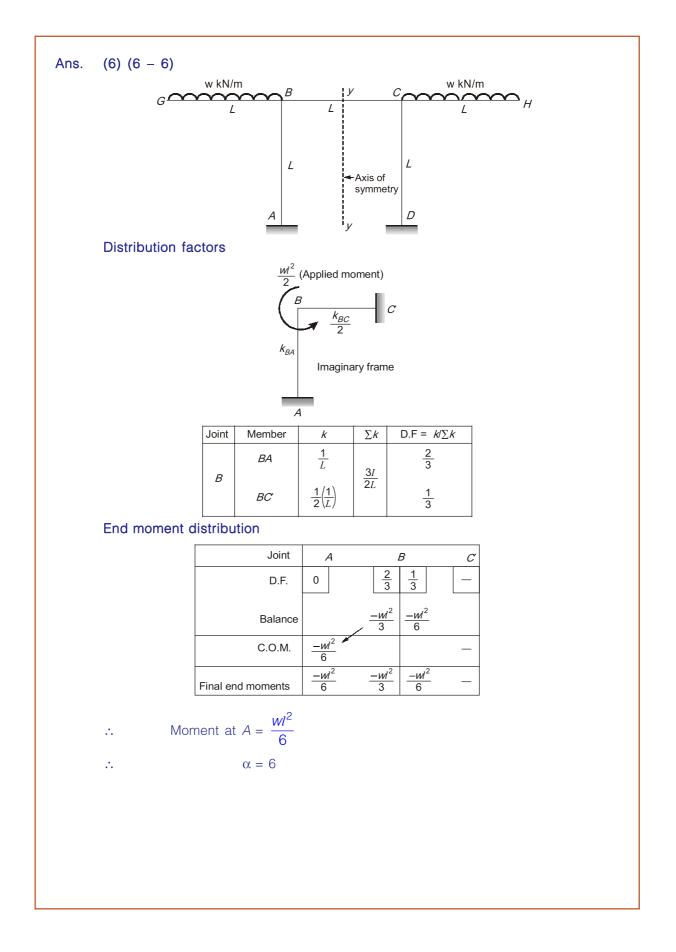
where x is in cm measured from A to B with x = 0 at A, t is in s,  $C_n$  are constants in °C, T is in °C, and  $\beta$  is in  $s^{-1}$ . The value of  $\beta$  (in  $s^{-1}$ , rounded off to three decimal places) is \_\_\_\_\_\_.

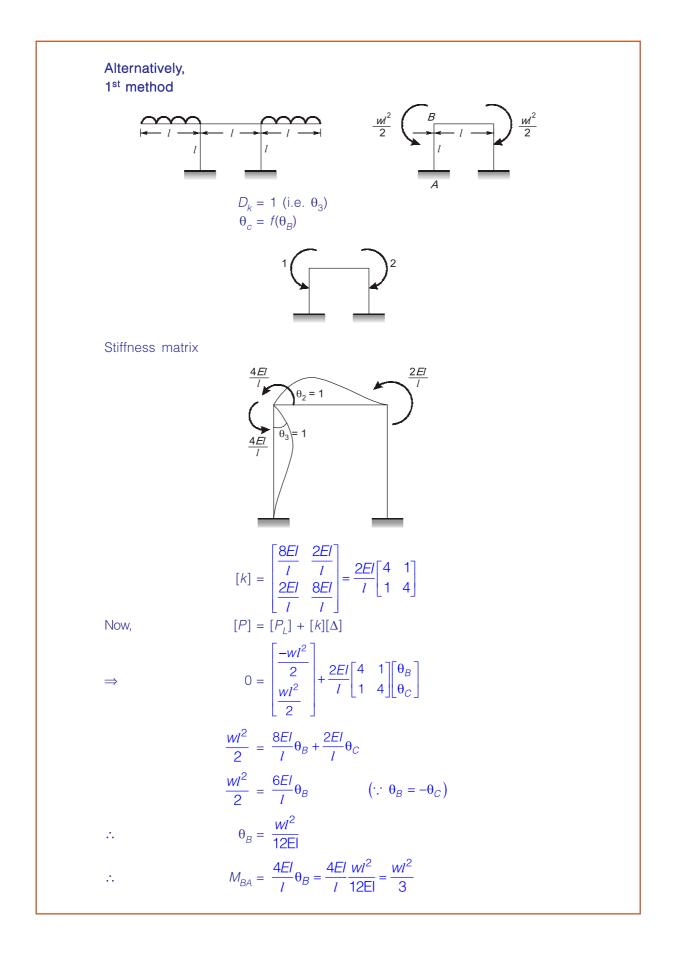
Ans. (0.395) (0.394 - 0.396) $\frac{\partial T}{\partial t} = \Delta \frac{\partial^2 T}{\partial x^2}; \quad T(0, t) = 0, \quad T(5, t) = 0, \quad T(x, 0) = T_0, \quad D = 1$ ...(1) Put D = 1, it's general solution using separation of variables methods is  $T(x, t) = (C_1 \cos px + C_2 \sin px)C_3 e^{-p^2 t}$ Using  $T(0, t) = 0 \implies C_1 = 0$ ...(2) Using  $T(5, t) = 0 \implies C_2 C_3 \sin 5p e^{-p^2 t} = 0$  $\sin 5p = 0 \implies p = \frac{n\pi}{5}, n \in I$ or  $T(x, t) = b_n \sin\left(\frac{n\pi x}{5}\right) \cdot e^{\frac{-t^2 \pi^2 t}{25}} \text{ where } b_n = C_2 C_3$ Hence, by (2), Hence most general solution is  $T(x, t) = \sum_{n=1}^{\infty} b_n \sin\left(\frac{n\pi x}{5}\right) e^{\frac{-n^2 \pi^2 t}{25}}$ ...(3) Now using  $T(x, 0) = T_0$  $T_0 = \sum_{n=1}^{\infty} b_n \sin\left(\frac{n\pi x}{5}\right)$  $\Rightarrow$ which is half range fourier sine series for  $T_0$  $b_n = \frac{2}{5} \int_0^5 T_0 \sin\left(\frac{n\pi x}{5}\right) dx = \begin{cases} 0, & n \text{ even} \\ \frac{4T_0}{n}, & n \text{ odd} \end{cases}$ Hence,  $T(x, t) = \sum_{n = orded} \frac{4T_0}{n\pi} \sin\left(\frac{n\pi x}{5}\right) e^{\frac{-n^2\pi^2 t}{25}}$ By (3), ...(4) On comparison with,  $T(x, t) = \sum_{n=1,3,5} C_n \sin\left(\frac{n\pi x}{5}\right) e^{-\beta n^2 t}$  $C_n = \frac{4T_0}{n\pi}$  and  $\beta = \frac{\pi^2}{25} = 0.3947 \approx 0.395$  $\Rightarrow$ End of Solution



**Q.54** For the frame shown in the figure (not to scale), all members (AB, BC, CD, GB, and CH) have the same length, *L* and flexural rigidity, *EI* The joints at B and C are rigid joints, and the supports A and D are fixed supports. Beams GB and CH carry uniformly distributed loads of *w* per unit length. The magnitude of the moment reaction at *A* is  $wL^2/k$ . What is the value of *k* (in integer)?







....

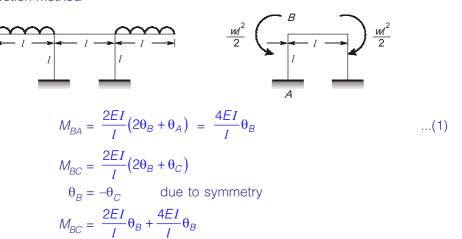
...

 $\Rightarrow$ 

....

$$M_{BA} = \frac{M_{BA}}{2} = \frac{Wl^2}{6}$$

2<sup>nd</sup> method Slope deflection method



Joint equation comliting

$$M_{BA} + M_{BC} = \frac{wl^2}{2}$$

$$\frac{6EI}{l}\theta_B = \frac{wl^2}{2}$$

$$\theta_B = \frac{+wl^2}{12EI}$$

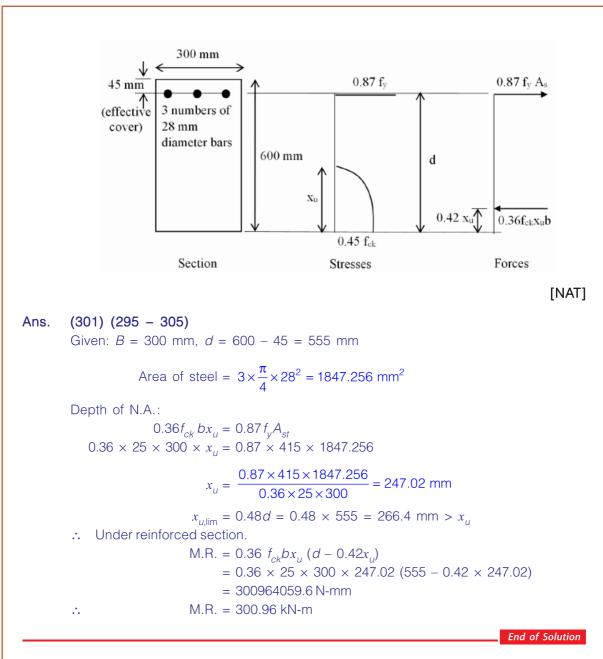
$$M_{BA} = \frac{4EI}{l} + \frac{wl^2}{12EI} = \frac{+wl^2}{3EI}$$

$$M_B = \frac{M_{BA}}{2} = \frac{M_{BA}}{2} = \frac{wl^2}{6EI}$$

End of Solution

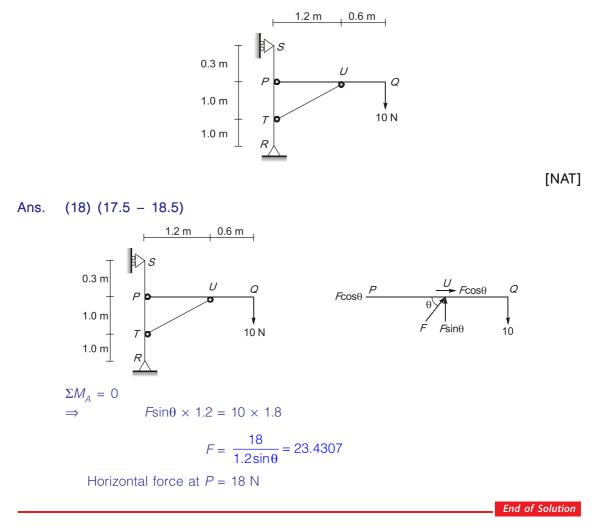
Q.55 Consider the singly reinforced section of a cantilever concrete beam under bending, as shown in the figure (M25 grade concrete, Fe415 grade steel). The stress block parameters for the section at ultimate limit state, as per IS 456 : 2000 notations, are given. The ultimate moment of resistance for the section by the Limit State Method is \_\_\_\_\_\_ kN.m (round off to one decimal place).

[Note: Here, As is the total area of tension steel bars, *b* is the width of the section, *d* is the effective depth of the bars,  $f_{ck}$  is the characteristic compressive cube strength of concrete,  $f_v$  is the yield stress of steel, and  $x_u$  is the depth of neutral axis.]



**Q.56** A 2D thin plate with modulus of elasticity,  $E = 1.0 \text{ N/m}^2$ , and Poisson's ratio,  $\mu = 0.5$ , is in plane stress condition. The displacement field in the plate is given by  $u = Cx^2y$ . and v = 0, where u and v are displacements (in m) along the X and Y directions, respectively, and C is a constant (in m<sup>-2</sup>). The distances x and y along X and Y, respectively, are in m. The stress in the X direction is  $\sigma_{XX} = 40xy \text{ N/m}^2$ , and the shear stress is  $\tau_{XY} = \alpha x^2 \text{ N/m}^2$ . What is the value of  $\alpha$  (in N/m<sup>4</sup>, in integer)?

Ans. (5) (5 - 5)We know that,  $\sigma_{x} = \frac{E}{(1-\mu^{2})} (\in_{x} + \mu \in_{y})$  $\sigma_{x} = \frac{E}{(1-\mu^{2})} \left( \frac{\partial u}{\partial x} + \mu \frac{\partial v}{\partial y} \right)$  $\sigma_x = \frac{E}{(1-\mu^2)} (2Cxy + 0)$  $\sigma_x = \frac{2ECxy}{\left(1-\mu^2\right)}$  $\sigma_x = 40xy \text{ N/m}^2$ Given,  $\frac{2EC_{x}y}{(1-\mu^2)} = 40_{x}y$  $\Rightarrow$  $C = \frac{40(1-\mu^2)}{2E} = \frac{40(1-0.5^2)}{2\times 1} = 15$  $\Rightarrow$  $\gamma_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}$ Now,  $\gamma_{xy} = Cx^2 + 0$  $G = \frac{E}{2(1+\mu)} = \frac{\tau_{xy}}{\gamma_{xy}}$ Also,  $\tau_{xy} = \frac{Cx^2 \times 1}{2(1+0.5)} = \left(\frac{15}{2 \times 1.5}\right)x^2 = 5x^2$  $\Rightarrow$ : On comparing with  $\tau_{xy} = \alpha x^2$ , the value of  $\alpha$  is 5. End of Solution Q.57 An idealised frame supports a load as shown in the figure. The horizontal component of the force transferred from the horizontal member PQ to the vertical member *RS* at *P* is \_\_\_\_\_ N (round off to one decimal place).

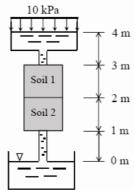


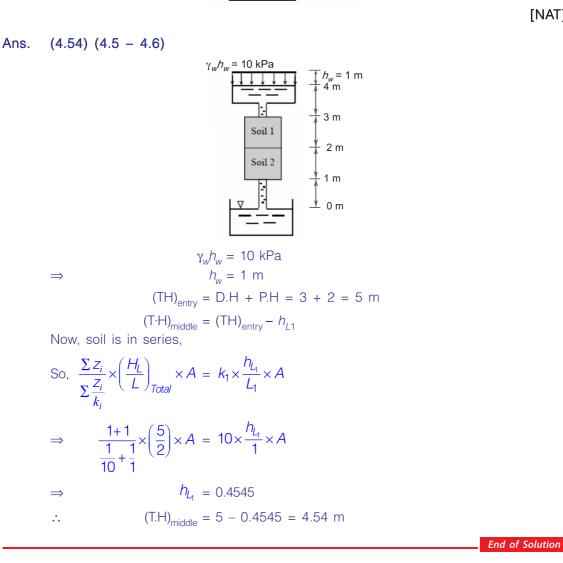
Q.58 A square footing is to be designed to carry a column load of 500 kN which is resting on a soil stratum having the following average properties: bulk unit weight = 19 kN/m<sup>3</sup>; angle of internal friction = 0° and cohesion = 25 kPa. Considering the depth of the footing as 1 m and adopting Meyerhoff's bearing capacity theory with a factor of safety of 3, the width of the footing (in m) is \_\_\_\_\_\_ (round off to one decimal place) [Assume the applicable shape and depth factor values as unity; ground water level at greater depth.]

Ans. (3.4) (3 - 3.4)As per Meyerhoff's theory,  $q_{ii} = CN_cS_cd_ci_c + qN_qS_qd_qi_q + 0.5\gamma BN_\gamma S_\gamma d_\gamma i_\gamma$ For  $\phi = 0$ ,  $N_c = 5.14$ ,  $N_q = 1$  and  $N_{\gamma} = 0$ Also, considering shape, depth and inclination factor as 1, we get  $q_{ii} = 5.14 \times 25 + \gamma D_f$  $\Rightarrow$  $q_{\mu\mu} = q_{\mu} - \gamma D_f = 5.14 \times 25$  $\Rightarrow$  $q_{ns} = \frac{q_{nu}}{\text{FOS}} = \frac{5.14 \times 25}{3} = 42.83 \text{ kN/m}^2$  $\Rightarrow$  $Q_{ns} = 500 \text{ kN} = q_{ns} \times B^2$ Also,  $42.83 \times B^2 = 500$  $\Rightarrow$ B = 3.41 m $\Rightarrow$ End of Solution Q.59 A circular pile of diameter 0.6 m and length 8 m was constructed in a cohesive soil stratum having the following properties: bulk unit weight = 19 kN/m<sup>3</sup>; angle of internal friction =  $0^{\circ}$  and cohesion = 25 kPa. The allowable load the pile can carry with a factor of safety of 3 is \_\_\_\_\_ kN (round off to one decimal place). [Adopt: Adhesion factor,  $\alpha = 1.0$  and Bearing capacity factor,  $N_c = 9.0$ ] [NAT] Ans. (146.9) (145 - 149) $Q_{\rm up} = 9c\left(\frac{\pi}{4}D^2\right) + \alpha \overline{c}(\pi DL)$ =  $9 \times 25 \left( \frac{\pi}{4} \times (0.6)^2 \right) + 1 \times 25 (\pi \times 0.6 \times 8)$  $Q_{\rm up} = 440.608 \text{ kN}$ *.*..

:. Allowable load = 
$$Q_{ap} = \frac{Q_{up}}{FOS} = \frac{440.608}{3} = 146.87 \text{ kN}$$

Q.60 For the flow setup shown in the figure (not to scale), the hydraulic conductivities of the two soil samples, Soil 1 and Soil 2, are 10 mm/s and 1 mm/s, respectively. Assume the unit weight of water as 10 kN/m<sup>3</sup> and ignore the velocity head. At steady state, what is the total head (in m, rounded off to two decimal places) at any point located at the junction of the two samples?





**Q.61** A consolidated drained (CD) triaxial test was carried out on a sand sample with the known effective shear strength parameters, c' = 0 and  $\phi' = 30$ . In the test, prior to the failure, when the sample was undergoing axial compression under constant cell pressure, the drainage valve was accidentally closed. At the failure, 360 kPa deviatoric stress was recorded along with 70 kPa pore water pressure. If the test is repeated without such error, and no back pressure is applied in either of the tests, what is the deviatoric stress (in kPa, in integer) at the failure?

## Ans. (500) (500 - 500)

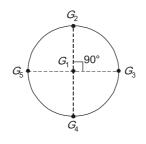
	$\overline{\sigma}_1 = \overline{\sigma}_3 \tan^2 \left( 45 + \frac{\phi}{2} \right) + 2c' \tan \left( 45 + \frac{\phi}{2} \right)$	
	$\overline{\sigma}_3 = \sigma_3 - 70$	
	$\overline{\sigma}_1 = \sigma_1 - 70 = \sigma_3 + \sigma_d - 70 = (\sigma_3 + 360 - 70)$	
$\Rightarrow$	$(\sigma_3 + 360 - 70) = (\sigma_3 - 70) \tan(45 + 30/2)$	$[\because c = 0]$
$\Rightarrow$	$\sigma_3 = 250 \text{ kPa}$	
Now,	$\sigma_1 = \sigma_3 \tan^2(45 + \phi/2)$	$[\because c = 0]$
$\Rightarrow$	$(\sigma_3 + \sigma_d) = \sigma_3 \tan^2 (45 + 30/2)$	
$\Rightarrow$	$(250 + \sigma_d) = 250 \times 3$	
$\Rightarrow$	$\sigma_d = 500 \text{ kPa}$	
		End of Solution

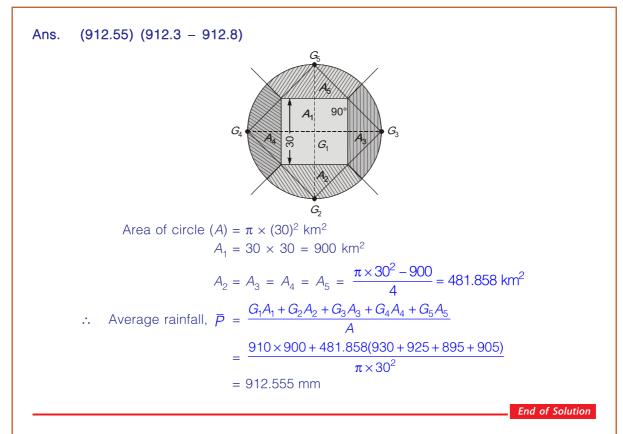
**Q.62** A catchment may be idealized as a circle of radius 30 km. There are five rain gauges, one at the center of the catchment and four on the boundary (equi-spaced), as shown in the figure (not to scale).

The annual rainfall recorded at these gauges in a particular year are given below.

Gauge	G1	G <sub>2</sub>	G <sub>3</sub>	<i>G</i> <sub>4</sub>	<i>G</i> <sub>5</sub>
Rainfall (mm)	910	930	925	895	905

Using the Thiessen polygon method, what is the average rainfall (in mm, rounded off to two decimal places) over the catchment in that year?





Q.63 The cross-section of a small river is sub-divided into seven segments of width 1.5 m each. The average depth, and velocity at different depths were measured during a field campaign at the middle of each segment width. The discharge computed by the velocity area method for the given data is \_\_\_\_\_ m<sup>3</sup>/s (round off to one decimal place).

Segment		Velocity (m/s) at different depths			
eegineitt	(m)	0.2 D	0.6 D	0.8 D	
1	0.40		0.40		
2	0.70	0.76		0.70	
3	1.20	1.19		1.13	
4	1.40	1.25		1.10	
5	1.10	1.13		1.09	
6	0.80	0.69		0.65	
7	0.45		0.42		

Ans. (8.5) (8.4 - 8.6)

....

		Velocity of depths			0.2D + 0.8D	A.,
Segment (1)	Average depth (D) m (1)	0.2 D (3)	0.6 D (4)	0.8 D (5)	2 (6)	Average Velocity
1	0.40		0.40			0.4
2	0.70	0.76		0.70	0.73	0.13
3	1.20	1.19		1.13	1.16	1.16
4	1.40	1.25		1.10	1.175	1.175
5	1.10	1.13		1.09	1.11	1.11
6	0.80	0.69		0.65	0.67	0.67
7	0.45		0.42			0.42

Effective width of 1<sup>st</sup> section = 
$$(\overline{W}_1) = \frac{\left(\frac{W_1 + \frac{W_2}{2}}{2W_1}\right)^2}{2W_1} = \frac{\left(\frac{1.5 + \frac{1.5}{2}}{2 \times 1.5}\right)^2}{2 \times 1.5} = 1.6875 \text{ m}$$

Effective width of 2<sup>nd</sup> to 6<sup>th</sup> section ( $W_2 = W_2 \dots = W_6$ )

$$=\left(\frac{1.5+1.5}{2}\right)=1.5\,\mathrm{m}$$

Effective width of 7<sup>th</sup> section =  $\frac{\left(1.5 + \frac{1.5}{2}\right)^2}{2 \times 1.5} = 1.6875 m$ 

Discharge = 
$$\sum_{i=1}^{7} \Delta Q_i \sum_{i=1}^{7} \Delta Q_i = \sum_{i=1}^{7} W_i \times y_1 \times \overline{V_i}$$

 $= (1.6875 \times 0.4 \times 0.4) + (1.5 \times 0.7 \times 0.73) + (1.5 \times 1.2 \times 1.16) + (1.5 \times 1.4 \times 1.175) + (1.5 \times 1.1 \times 1.11) + (1.5 \times 0.8 \times 0.67) + (1.6875 \times 0.45 \times 0.42) = 8.564 \text{ m}^3/\text{sec}$ 

**Q.64** The theoretical aerobic oxidation of biomass (C<sub>5</sub>H<sub>7</sub>O<sub>2</sub>N) is given below:  $C_5H_7O_2N + 5O_2 \rightarrow 5CO_2 + NH_3 + 2H_2O$ 

The biochemical oxidation of biomass is assumed as a first-order reaction with a rate constant of 0.23/d at 20°C (logarithm to base e). Neglecting the second-stage oxygen demand from its biochemical oxidation, the ratio of BOD<sub>5</sub> at 20°C to total organic carbon (TOC) of biomass is \_\_\_\_\_\_ (round off to two decimal places).

[Consider the atomic weights of C, H, O and N as 12 g/mol, 1 g/mol, 16 g/mol and 14 g/mol, respectively]

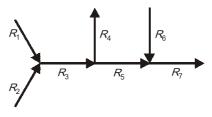
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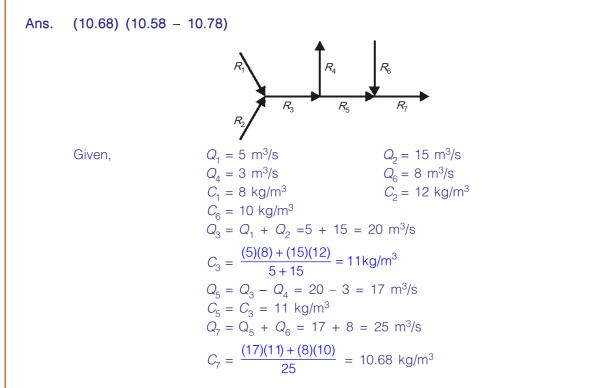
End of Solution

## Ans. (1.82) (1.8 – 2)

$$\begin{split} & C_5H_7O_2N + 5O_2 \rightarrow 5CO_2 + NH_3 + 2H_2O \\ \text{Molar mass of } C_5H_7O_2N = 113g \\ & \text{TOC} = 5 \times 12 = 60g \\ \text{Ultimate BOD} = 5 \times 32 = 160g \\ & \text{BOD}_5(20^\circ\text{C}) = 160(1 - e^{-0.23 \times 5}) \\ & \text{BOD}_5(20^\circ\text{C}) = 109.34g \\ & \frac{BOD_5(20^\circ\text{C})}{TOC} = \frac{109.34}{60} = 1.822 \end{split}$$

**Q.65** A system of seven river segments is shown in the schematic diagram. The  $R_i$ 's,  $Q_i$ 's, and  $C_i$ 's (i = 1 to 7) are the river segments, their corresponding flow rates, and concentrations of a conservative pollutant, respectively. Assume complete mixing at the intersections, no additional water loss or gain in the system, and steady state condition. Given:  $Q_1 = 5 \text{ m}^3/\text{s}$ ;  $Q_2 = 15 \text{ m}^3/\text{s}$ ;  $Q_4 = 3 \text{ m}^3/\text{s}$ ;  $Q_6 = 8 \text{ m}^3/\text{s}$ ;  $C_1 = 8 \text{ kg/m}^3$ ;  $C_2 = 12 \text{ kg/m}^3$ ;  $C_6 = 10 \text{ kg/m}^3$ . What is the steady state concentration (in kg/m<sup>3</sup>, rounded off to two decimal place) of the pollutant in the river segment 7 ?





End of Solution